

HUNTER SUBDIVISION SERVICING FEASIBILITY STUDY

June 1, 2022

Prepared for: Auburn Developments Inc.

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1 INTRODUCTION

1.1 PURPOSE OF REPORT

This report has been prepared for the information of Auburn Developments Inc. to provide background information and servicing strategy on the development of the property located at 1598 Richmond Street in the Town of Dorchester in accordance with the Draft Plan of Subdivision, included in Appendix 'A'. This report provides general background information regarding the property, its preliminary servicing information including discussions of storm servicing and the provision of stormwater management, sanitary servicing and connection to existing downstream sewers, and connection to the Town of Dorchester's municipal water supply system.

1.2 LOCATION

The lands are municipally addressed as Mun No. 1598 Richmond Street in the Town of Dorchester. The area of the subject site is approximately 44 hectares (108.6 acres), and the site is situated directly north of the CN railway tracks, directly south of Marion Street, east of Linwood Drive, and west of Clara Street. The site is located within the Dorchester Settlement Area of the Municipality of Thames Centre Official Plan.

2 LIMITATION OF REPORT

The information presented in this report is based on the review of the following information:

- As-constructed drawings on file with the Municipality of Thames Centre;
- Municipality of Thames Centre Engineering Design Standards and Specifications;
- The Hunter Property Draft Plan of Subdivision as prepared by Stantec Consulting Ltd.;
- Preliminary Geotechnical Investigation as prepared by EXP Services Inc., dated June 2021;
- Hydrogeological Assessment (Preliminary Report) as prepared by EXP Services Inc., dated January 19, 2022;
- Municipality of Thames Centre 2021 Development Charges Background Study: and,
- Municipality of Thames Centre Water and Wastewater Master Plan Update Master Plan Report, prepared by GM BluePlan, dated August 2019.

3 Site Characteristics

3.1 Existing Land Use and Topography

The site is undeveloped at the present time, with the exception of the existing farm buildings located east of Richmond Street. There are five buildings located on the site, which appear to be a dwelling unit and accessory structures, based on aerial photos. The remaining property has been used for agricultural purposes, primarily crop production up to the present time.

Site elevations range from approximately 273 metres in the northeast corner to approximately 255 metres along the southern property line. The site topography generally slopes downwards towards the CN Railway tracks, but there is a significant ridge that is oriented in the east-west direction. While the average site slope is approximately 2%, there are portions of the ridge with slopes approaching 13%.

An initial analysis of the existing grades indicates that the existing topography does not preclude development on any portion of the property. However, substantial earthworks will likely be required in localized areas to flatten the east-west ridge.

3.2 Site Access

The northern property line borders Marion Street, with approximately 1050 m of frontage. Richmond Street crosses the property and provides approximately 400 m of frontage to both west and east lands. Additionally, the Middlesex County mapping shows unopened road allowances that extend westward into the subject site from Ida Street, Eva Street, and Minnie Street, which could provide access to the property from Clara Street.

4 Sanitary Servicing

4.1 Existing Sanitary Services

There is currently no sanitary sewer servicing for the subject site, and no existing local sanitary sewers crossing of the Thames River.

The existing Dorchester wastewater treatment plant (WWTP) is located on the south side of the Thames River near the Dorchester Fairgrounds.

4.2 Proposed Sanitary Servicing

The most recently completed sanitary servicing study that includes the subject lands is the Municipality of Thames Centre Water and Wastewater Master Plan Update – Master Plan Report, prepared by GM BluePlan and dated August 2019.

In order for the subject lands to develop, the site would require the sanitary sewer system to be constructed in accordance with the Municipality of Thames Centre Water and Wastewater Master Plan (2019) preferred strategy for sanitary servicing of the North Dorchester area, much of the contemplated capital works would be within its boundary. The preferred strategy would establish a new North Dorchester Sanitary Pump Station (SPS) which would provide the necessary sanitary outlet for the proposed gravity sewers and forcemain from surrounding developments including subject site (illustrated in Figure 1 & Figure 2 below). The subject site is therefore instrumental and likely the first development trigger required to move forward with the preferred strategy for servicing North Dorchester.

Refer to Appendix 'A' for the subject site proposed preliminary sewer routing details, including depth and how the intended lands which include lands external to the proposed development can ultimately flow by way of a gravity sewer to the proposed North Dorchester Sanitary Pump Station.



Figure 1. North Dorchester Preferred Sanitary Servicing Strategy as per Updated Master Plan (2019)



Figure 2. North Dorchester Preferred Sanitary Servicing Strategy Capital Works as per Updated Master Plan (2019)

The following is a list of anticipated capital works required to service the entirety of the subject site as identified in the Updated Master Plan along with accompanying estimated costs (2021 Estimate) and timing as per The Municipality of Thames Centre 2021 Development Charges Background Study:

- North Dorchester New Development Sanitary Pump Station (WW-D-SPS-01B) \$5,400,000 Timing 2030
- North Dorchester New Development Forcemain (WW-D-FM-01) New 300mm forcemain needed to support new development SPS for development blocks North of CN rail in North Dorchester – \$1,418,500 – Timing 2030
- Dorchester Road Forcemain extension (WW-D-FM-02) New 300mm forcemain from Dorchester Road bridge forcemain (Jane St) to Dorchester Road gravity sewers needed to support new

development SPS for development blocks North of CN rail in North Dorchester - \$415,100 - Timing 2026

- Dorchester Road Forcemain extension (WW-D-FM-03) New 300mm forcemain from Dorchester Road bridge forcemain (Jane Street) to Byron Ave 600mm diameter trunk sewers needed to support new development SPS for development blocks North of CN rail in North Dorchester – 1,013,700 – Timing 2026
- Richmond Street Sewer (WW-D-SS-16) New 200mm diameter sanitary sewer to accommodate growth Northeast of new North Dorchester SPS \$587,300 Timing 2037
- New Development SPS East Sewers (WW-D-SS-02) new 250mm diameter sanitary sewer required for development blocks going to new Development SPS in North Dorchester. - \$811,900 – Timing 2030

The capital works listed above total estimated cost is \$10,205,300.

The wastewater capital program (partially listed herein) will work as a foundation for the Municipality of Thames Centre's Capital Budget. The capital program provides a list and timing of new assets that the Municipality will have to operate and maintain; and therefore, it is the starting point for the planning of operation and maintenance costs and resources allocation for new wastewater infrastructure.

The funding source and timing was explicitly stated in the Municipality of Thames Centre 2021 Development Charges Background Study, however the timing for works can be suggested to coincide with the new development and based on the projected population and employment growth within the Municipality of Thames Centre. The funding (including availability) and timing of the infrastructure required for the Hunter Subdivision to proceed would have to be confirmed in discussion with the Municipality of Thames Centre.

4.3 Wastewater Treatment Plant Capacity

The existing Dorchester WWTP is located on the south side of the Thames River and uses a sequencing batch reactor secondary treatment process. The existing plant capacity is approximately 900 m³/d and expected to achieve 1,200 m³/d in 2022 with planned expansion. In 2017 only 56% of 520 m³/d rated capacity at the time was utilized leaving substantial room for growth contribution.

The expansion to 1200 m³/d is a part of the long-term plan to expand the WWTP to an ultimate capacity of 6,000 m³/d, as indicated in the original Certificate of Approval (CofA) issued in June 1999, which was later amended in November 2000 for approval for the 2002 capacity expansion to 520 m³/d.

4.4 Proposed Sewage Flow

We have assumed a design flow of 350 L/capita/day in calculating the anticipated subdivision sewage flow, which is consistent with current Municipality of Thames Centre standards. For the purpose of calculating the sewage flows, the Municipality's standard population densities for various land types were used. These densities are summarized as follows:

Table 1. Land Use Densities (Municipality of Thames Centre)

Land Use	Projected Density
Low Density Residential	30 units/ha @ 3 people/unit
Medium Density Residential	75 units/ha @ 2.4 people/unit

The estimated peak design flow for the full-build out of the Hunter Subdivision, using the above densities and the current Draft Plan of Subdivision, are detailed in the following table.

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Land Use	Area (ha)	Number of Units	Population
Low Density Residential	15.977	479	1437
Medium Density Residential	10.799	810	1944
Non-contributing Blocks and Rights-of-way	16.936	-	-
Total	43.712		3381

Total Peak Flow

45.32 L/s

As identified in the above table, the proposed site is approximately 44 ha which includes 479 single-family lots and 810 multi-family units for a total population of approximately 3381 persons. Based on an average daily flow of 350 L/capita/day, a total peak discharge of approximately 45.32 L/s is estimated for the entire development including infiltration allowance, and Harmon peaking factor.

5 Stormwater Management

5.1 Existing Site Drainage

The existing condition of the site is currently agricultural land. Various municipal drains cross through the site including Sandusky Drain, Hunter Branch, Porter Subdivision Drain and Hunt Drain, as shown in **Figure 3**.



Figure 3. Site Existing Condition

The subject site generally drains from east to west, splitting into six catchment areas with the majority draining towards Sandusky Drain. The Hunt McMillian Drain flows through a culvert in the CN railroad embankment, then through approximately 600 m of open channel before discharging into the Thames River. The future stormwater collection system must be designed to accommodate the flows from the existing municipal drains. Incorporation of these drains into the storm sewer system for the subdivision will require the completion of a drainage report.

5.2 Proposed Stormwater Management Strategy

The preferred alternative for stormwater management would be to create two stormwater management facilities (SWMF) on the north and south property limits on Parcel #2. The north SWMF will ultimately discharge into Sandusky Drain. The south SWMF will outlet to an open channel parallel to CN Railway and run west under Richmond Street via a proposed culvert and discharge into Sandusky Drain.

Parcel #1 (West of Richmond Street) will not have a SWM facility and instead will require on-site measures to provide quality control. Quantity control can be provided by over-controlling runoff in SWM facilities east of Richmond Street. The stormwater management strategy and preliminary design is available in the Appendix 'B' as detailed in the Conceptual SWM Strategy Memo prepared by Stantec Consulting Ltd., dated May 16, 2022.

6 Municipal Water Servicing

6.1 Existing Water Services

A municipal water tower is located southwest of the Marion Street/Clara Street intersection. There is an existing 250 mm diameter PVC watermain located on Marion Street, along the northern property.

Additionally, an existing 150mm diameter watermain on Linwood Drive within the neighboring Maplewoods Estates subdivision to the west is available for connection as well as an existing 150mm diameter watermain on Richmond Street just south of the CN Railway.

Lastly, a 250 mm diameter existing watermain is located along Clara Street, which feeds 150 mm diameter watermains on Ida Street, Eva Street, and Minnie Street. Watermain on Ida and Eva Street are expected to be extended into the subject site during capital infrastructure projects.

The Thames Centre as-built plan and profile information illustrating watermain location as detailed herein is included in Appendix 'A'.

6.2 Existing System Supply

According to the 2019 Water and Wastewater Master Plan Update (WWMP), the municipality is conducting well exploration for additional groundwater sources needed for the proposed build-out of Dorchester. In the event no suitable groundwater is found, a connection can be made to Lake Huron and Elgin Area Primary Supply Systems to provide needed capacity. In the meantime, upgrades to the pump and well at Dorchester Water Treatment Facility (WTF) are expected to provide sufficient capacity and pressure to attain peak hour and fire flow conditions to the subject site. Expected timeline for pump and well upgrades is 2023.

Dorchester is currently a single pressure zone supplied by 8 production wells that are pumped and treated at Dorchester WTF. Dorchester operates one elevated tank to provide system head pressure, and

two underground 2500 m³ reservoirs for storage and chlorine contact time used in the disinfection of groundwater.

Maximum annual permitted withdrawal volume is 2,681,086 m³ as per the Permit to Take Water (PTTW) issued by the Ministry of Environment, Conservation and Parks (MECP). The current PTTW is slated to expire in May 2021, but changes to permitted water use is not expected to change. Percentage of actual annual withdrawal under present conditions is 453,380 m³, which is 17% of total the allocated capacity.

If the assumed Average Day Demand (ADD) is taken to be 225L/cap/day (Table 2.1, Master Plan) with a maximum projected subdivision population of 3000 people, gives 675,000L/day (675 m³/day) or 246,375 m³/year. This conservative figure brings water utilization to 26.1% of the theoretical maximum capacity.

6.3 Proposed Water Servicing

It is understood per Municipality of Thames Centre Design Standards (2021) Water distribution systems ought to be designed so that no more than fifty (50) units with individual water services and meters are serviced from a single source of supply. Therefore, given the projected number of units for the proposed subdivision it is expected that secondary connection will be required for looping and supply redundancy.

It is anticipated available connections for new water service to the development will be at connection points as listed below. Ultimate watermain connection locations and sizes will be confirmed during detailed design and based on pressure and flow requirements of the proposed development.

- Water service connection for servicing proposed Linwood Drive from existing 150mm diameter watermain in Maplewoods Estates Subdivision
- Water service connection at Street A from existing 250mm diameter watermain on Marion Street
- Watermain extension of existing 150mm diameter watermain on Richmond Street north or alternatively extension from Marion Street 250mm watermain to service medium density block on Richmond Street and to provide secondary connection at Street B
- Watermain extension of existing 150mm diameter watermain on Ida Street
- Medium Density Blocks fronting Marion Street to be serviced off existing 250mm diameter watermain on Marion Street

7 Summary and Recommendations

Water servicing is generally available as is access to the boundary road network. The absence of sanitary servicing and stormwater management infrastructure in the vicinity of development will require the implementation of the proposed strategy as detailed herein to service the development.

This report concludes that a sanitary outlet can be obtained through a funding formula with the Municipality, however, the agreement of the municipality and a significant up-front cost is required to develop due to external work required and benefit to growth in North Dorchester Area.

If you have any questions or concerns regarding the enclosed information, we would be pleased to meet or discuss with you further.

Hunter Subdivision Servicing Feasibility Study

APPENDICES



Appendix A



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52.0 51.0		ASPHALT EXISTING & ROAD TIO.80%	AS CONSTRUCTED T/G 261.322 INV. W. 258.642 EXISTING FIN & ROAD	9.184	263.0
2.0 1.0 0.0		ASPHALT EXISTING & ROAD TIO. 50%	AS CONSTRUCTED T/G 2.61.322: INV. W. 258.642 EXISTINC FIN & ROAD	9.184 20116 LIMIT OF TURNING CIRCLE ESMT	263.0 262.0 261.0 260.0
2.0	250mme CB LEAD TO BE CONNECTED	ASPHALT EXISTING & ROAD tio.80% EXISTING & ROAD EXISTING & ROAD tio.80% EXISTING & ROAD EXISTING & ROAD E	AS CONSTRUCTED T/G 2/GI.322: INV. W. 258.642 EXISTING FIN & ROAD FIN & ROAD EXISTING I SOMME W/W PLUC	9.184 20116 LIMIT OF TURNING CIRCLE ESMT TYP. 10.10 10.10	263.0 262.0 261.0 260.0
52.0 51.0 50.0	250mme CB LÉAD TO BE CONNECTED TO STORM SEWER -	ASPHALT EXISTING & ROAD TIO.80% E D F Z S SOOmme STORM	AS CONSTRUCTED T/G 2.61.322 INV. W. 258.642 EXISTING FIN & ROAD EXISTING 150mme W/M LUG EXISTING 600mme STM	9.184 20116 LIMIT OF TURNING CIRCLE ESMT 7.5% 2011 10 10 2011 20	263.0 262.0 261.0 260.0 259.0
2.0 1.0 0.0 9.0 8.0	250mme CB LEAD TO BE CONNECTED TO STORY SEWER -	ASPHALT EXISTING Q HOAD TIO.80% E D E D T M E D E D E D E D E D E D E D E D	AS CONSTRUCTED T/G 2/61.322 INV. W. 259.642 EXISTING FIN & ROAD FIN & ROAD FIN & ROAD FIN & ROAD FIN & ROAD FIN & ROAD EXISTING STM EXISTING EXISTING STM EXISTING STM EXISTING STM	9.184 20116 LIMIT OF TURNING CIRCLE ESMT 2.5% 10 19 2.0m ROUNDINS FUTURE GUTER LINE	263.0 262.0 261.0 260.0 259.0 258.0
2.0 1.0 0.0 9.0 8.0	250mme CB LEAD TO BE CONNECTED TO STORY SEVER -		AS CONSTRUCTED T/G 2 GL 322 INV. W 258.642 EXISTING FIN & ROAD FIN & RO	9.184 20116 LIMIT OF PAVEMENT STRUCTURE. LIMIT OF TURNING CIRCLE ESMT CIRCLE ESMT CIRCLE ESMT CIRCLE ESMT CIRCLE SMT CIRCLE ESMT CIRCLE ES	263.0 262.0 261.0 260.0 259.0 258.0 257.0
2.0 1.0 0.0 9.0 8.0 7.0	ZSOmre CB LEAD TO BE CONNECTED TO STORY SEMER -	ASPHALT EXISTING & ROAD TIO.80% E D E D E D E D E D E D E D E D	AS CONSTRUCTEO T/G 250.322 INV. W. 250.642 FIN & ROAD FIN & ROAD FIN & ROAD FIN & ROAD FIN & ROAD FIN & ROAD FIN & ROAD EXISTING FIN & ROAD EXISTING EXIS	9.184 20116 LIMIT OF TURNING CIRCLE ESMT 200 CIRCLE ESMT 200 200 200 200 200 200 200 20	263.0 262.0 261.0 260.0 259.0 259.0 258.0 257.0
2.0 1.0 0.0 9.0 8.0 7.0 6.0	250mm CB LEAD TO BE CONNECTED TO STORY SEWER	ASPHALT EXISTING & ROAD tio.so% E B S S S S S S S S S S S S S	AS CONSTRUCTED TG 26(1322: INV. W. 256(42 INV. 25	9,194 9,194 Cincle ESMT Cincle ESMT Cincle ESMT Concle ESMT Conc	263.0 262.0 261.0 260.0 259.0 259.0 259.0 258.0 257.0 256.0
2.0 1.0 0.0 9.0 8.0 7.0 6.0	250mme CB LEAD TO BE CONNECTED TO STORE SEWIR -		A.S. CONSTRUCTED T/G 2.61.322: INV. W. 258.642	9,194 20116 LIMIT OF TURNING CIRCLE ESMT P. 5% 20116 LIMIT OF TURNING CIRCLE ESMT TYP. 10 10 2010 2010 10 10 2010	263.0 262.0 261.0 260.0 259.0 259.0 258.0 258.0 256.0
62.0 61.0 60.0 59.0 58.0 57.0 56.0	ZSOMME CB LEAD TO BE CONNECTED TO STORE SEWER -	ASPHALT EXISTING & ROAD tio.60% E 0 E 150mm# W/M ELUG 600mm# STORM	A 5 CONSTRUCTED T/G 2/6/322 INV. W 250642 INV. W 250642 EXISTING FIN & ROAD EXISTING EXISTING COMMENTER EXISTING COOMMENTER EXISTING COOMANTER EXISTING COOMANTER EXISTING COOMANTER EXIST	9,194 9,194 20116 LLIMIT OF TURNING CIRCLE ESWIT TYPE 10 10 2.5% V 2.0m ROUNDING FUTURE GUTER LINE SECTION 'A=A' NT.3. 10 10 10	263.0 262.0 261.0 260.0 259.0 259.0 258.0 258.0 257.0 256.0
262.0 261.0 259.0 259.0 257.0 257.0 256.0		ASPHALT EXISTING & ROAD tio.50% E 0 E 150mm# W/M ELUC 600mm# STORM CSA A257.2 CL 65-D, BEDDING AS PER 0.P.S.D. 802.03 CLASS B	AS CONSTRUCTED 7/G 28:382; IMV. W: 239.642 EXISTING FIN @ ROAD FIN @ ROAD EXISTING EXIST. TO SARADA EXIST. TO SARADA EXIST. FIN @ ROADA EXIST.	9.184 - LIMIT OF PAREMENT STRUCTURE, - LIMIT OF PAREMENT, - LIMIT OF PAREMENT, - LIMIT OF	263.0 262.0 261.0 260.0 259.0 259.0 258.0 258.0 258.0 258.0 258.0 258.0 256.0 256.0
262.0 261.0 260.0 59.0 58.0 57.0 56.0		ASPHALT EXISTING & HOAD TO.80% E E CSA A257.2 CL 65-D, BEDDING AS PER O.P.S.D. B02.03 CLASS B	EXISTING EXISTI		263.0 262.0 261.0 260.0 259.0 259.0 259.0 259.0 258.0 258.0 258.0 258.0 258.0 258.0 258.0 258.0 258.0 258.0 256.0 256.0
52.0 51.0 50.0 59.0 58.0 57.0 56.0		ASPHALT EXISTING & ROAD TO.BOYA E E E TO.BOYA E E E E E E E E E E E E E	EXISTING COMPANY ALUG EXISTING COMPANY ALUG	TOWNSHIP OF RTH DORCHESTER	263.0 262.0 261.0 261.0 260.0 259.0 259.0 258.0 258.0 258.0 257.0 256.0 TC- 175 SMATING TC- 175 SMATING TC- 175 SMATING MALEXA MURLIST 19.02 (R.E.M. STORM S



Appendix B