

Draft Functional Servicing Report

SE Watson Farm Condo Development
21829 Nissouri Road, Thorndale, Ontario

Project Number: DEL19-103

Date: June 16, 2023

Prepared For:

1732435 Ontario Ltd.



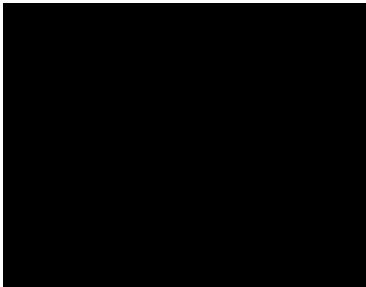
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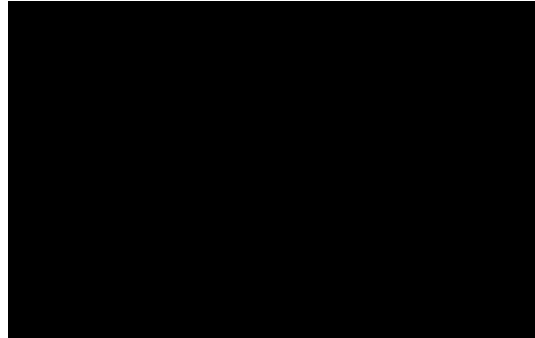
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Revisions

Original Report, dated: June 16, 2023

Statement of Qualifications and Limitations

This Report has been prepared for 1732435 Ontario Ltd. based upon currently available information, to support the Site Plan Application to be filed for the proposed Watson Farm Condominium Development. This report has been prepared for the sole use of 1732435 Ontario Ltd. and any reliance upon this information by any third party is made at the risk of that party. Development Engineering (London) Limited assumes no liability for any injury, loss or damage suffered by any third party based upon decisions made or actions that arise due to that party's reliance upon or interpretation of the contents of this report.

This Statement of Qualifications and Limitations is attached to and forms part of the Report; any use of the Report is subject to the terms contained herein.

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1.0 Introduction

Development Engineering (London) Limited, *DevEng*, has been retained by 1732435 Ontario Inc. to undertake engineering design for a proposed 91-unit condo development referred to as Watson Farm condo development at 21829 Nissouri Road. The subject site is comprised of a +/- 3.2 ha agricultural parcel in the southeast part of the existing Watson Farm, bound by Thorndale Road to the south, Nissouri Road to the east, existing residential to the southeast, and existing agricultural to the north and west. The subject site is described by AGM Legal Survey plan 8-L-5871-Z (June 09, 2022) as Part of Lot 16, Concession 2, Geographic Township of West Nissouri, in the Municipality of Thames Centre.

The existing topography within the development area is undulating but ultimately slopes west directing runoff into the existing farm field under higher storm events, also owned by the applicant, with geodetic elevations across the site ranging between $\pm 284.75\text{m}$ along Nissouri Road in the east, down to $\pm 282.20\text{m}$ along the existing farm field to the west.

The condo development is proposed to be serviced with new watermain, sanitary sewers, storm sewers, and a dry basin/infiltration gallery stormwater management facility (SWMF). Road access to the site is proposed from two points through a proposed extension of Elliott Trail West, which would connect to Nissouri Road at the north limit of the site. Emergency access is to be available from Thorndale Rd. at the south limit per County of Middlesex requirements.

The purpose of this Functional Servicing Report is to support Site Plan Approval by demonstrating an effective approach to address servicing, grading, and stormwater management, while also showing due regard for municipal and provincial design guidelines for this private condo development.

2.0 Background Information

2.1 Previous Consultations

- Pre-consultation with the County of Middlesex (November 05, 2020).

2.2 Previous and Concurrent Studies and Reports

- Geotechnical Investigation – Watson Farm Development (EXP Services Inc., Nov. 2020).
- Hydrogeological Investigation – Watson Farm Aggregate Resources Study (EXP Services Inc., Jan 2016).
- Noise Feasibility Study – (Howe Gastmeier Chapnik Ltd., Jan 2023).
- Stage 1-2 Archaeological Assessment (Lincoln Environmental Consulting Corp., May 2020).
- McGuffin Drain Plan and Profile (Spriet Associates, 1979).

2.3 Related Design Specifications

- Municipality of Thames Centre Design Standards (2021).
- Ontario Building Code.
- MECP SWM Design Guidelines (2003).

3.0 Approval Agencies

3.1 Municipality of Thames Centre

The subject site is located with the Municipality of Thames Centre, Ward of West Nissouri. The condo site plan and engineering design must be submitted to and subsequently approved by the Municipality and County of Middlesex. The Municipality will also be responsible for the approval of any zoning by-law amendments and building permits associated within the subject site (private servicing, residential units).

3.2 County of Middlesex

The subject site is located within the County of Middlesex. The Plan of Condominium Application must be submitted to and subsequently approved by the County.

3.3 Ministry of Environment, Conservation, and Parks

The proposed private stormwater management facility (SWMF) and infiltration gallery may require ECA approval subject to further agency discussion. If required, an Environmental Compliance Approval (ECA) submission to the Ministry would be submitted during the detailed engineering design stage unless the Municipality agrees to approve servicing under Part 7 OBC as a private condo drainage system.

4.0 Existing Conditions

As discussed in Section 1.0, under existing conditions, the 3.2 ha undeveloped site (C=0.20; 0% imperviousness) is currently bound by Thorndale Road to the south, Nissouri Road to the East, existing residential to the southeast, and existing agricultural farmland to the north and west. Primary access to the site is to be provided from an extension of Elliott Trail (West of Nissouri Road) to the north.

4.1 Site Topography and Drainage

Review of the topo survey indicates the 3.2 ha site appears to be rolling at approximately 1.2% from +/-284.50m in the northwest corner to +/-283.5m in the centre of the site. The existing field currently pools runoff in the southwest portion of the site. The existing field is currently undulating but in larger storm events, the runoff bypassing the existing sag would tend to spill west toward the McGuffin drain which crosses the Watson farm. In consideration of a proposed aggregate operation on this farm, the proposed ground conditions at pit rehabilitation would direct such flow toward an infiltration pond located at the far west end of the property. Based on the pit grading strategy, there would be ample available active storage for runoff below original ground.

4.2 Soil Conditions

A geotechnical study was completed for the proposed development by EXP Services Inc. in November of 2020 based on six (6) boreholes drilled in March of 2020. Following the completion of the boreholes, three (3) monitoring wells were strategically installed across the site to observe groundwater conditions over a twelve (12) month period.

The site contains a surficial topsoil layer ranging from 250 to 300mm thick. The underlying soils primarily consist of sand and gravel with three (3) of the boreholes containing a layer of clayey silt/sandy silt.

Groundwater was encountered at depths of 1.8 and 3.0 mbgs. The groundwater monitoring program indicates the general direction of groundwater flow is to the south and is generally expected to be influenced by a nearby fluvial terrace.

On-site testing was conducted to confirm the hydraulic conductivity of native subsoils. The resulting infiltration rates ranged from 130mm/hr for the sand and between 80 to 135 mm/hr for the sand and gravel, indicating that the native subsoils are highly conducive to infiltration.

A gas detection meter was used on-site to test the presence of methane. It was determined that no methane gas producing materials or significant organic matter were present within the boreholes, except a thin veneer of topsoil. No significant methane gas concentration was detected in the boreholes.

Based on the information provided from the boreholes/monitoring wells, near surface soils encountered at some of the test hole locations have a potential for use in LID stormwater management or recharge design.

Refer to the Geotechnical Investigation report (EXP Project Number LON-00017870-GE) dated November 25, 2020 for more information.

4.3 Existing Roads

The existing roadways surrounding the site are Thorndale Road (County Road 28) to the south, and Nissouri Road to the east. Condo access is proposed from a new extension of Elliott Trail West from Nissouri Road. An emergency access is also proposed from Thorndale Rd. at the south limit per County requirements but will be blocked off from regular vehicle traffic with bollards and a chain.

4.4 Existing Servicing

Based on our review of the records identified above, the following observations have been made regarding servicing in proximity to the site:

- There is an existing municipal 200mm dia. PVC watermain on Thorndale Road to the south as well as a 150mm dia. PVC watermain on Nissouri Road to the east.
- There is an existing 300mm diameter sanitary sewer located on Thorndale Road to the south available to service the condo.
- There are no existing storm sewers within the vicinity to service the proposed development. Local road side ditches are located on both sides of Thorndale Road to the south and Nissouri Road to the east but these are not to be used for drainage outlet as they are too shallow for service.
- The existing McGuffin Drain tile traverses the Watson farm to the northwest of this condo site. It currently provides outlet for field drainage systems.

5.0 Proposed Conditions

The proposed scope of work includes a 91-unit condominium development, comprising of: five (5) private roads, various parking areas, a common element area, a private SWMF (dry basin), and a public road extension (Elliott Trail West).

5.1 Roads

Vehicular access to the subdivision is proposed with connection to two existing roadway systems: Thorndale Road (County Road 28) to the south for emergency access, and a proposed street extension (Elliott Trail West) connecting to Nissouri Road to the east. Approximately 570m of private, local streets are proposed for this

development. Best efforts have been implemented to align east bound traffic from the proposed Elliott Trail West extension with the ROW across Nissouri Road. The intersection is proposed as a stop condition and a terminal bulb is proposed at the west end for ease of road maintenance.

Design of the local streets will be completed in accordance with the recommended pavement structure in the Geotechnical Report by EXP Services Inc. The associated pavement design for local traffic has been established as follows:

- 40mm HL-3 top course asphalt;
- 50mm HL-8 base course asphalt;
- 150mm Granular 'A' Base; and,
- 300mm Granular 'B' Base.

The County Road Tie-ins require pavement design which includes:

- 50mm HL-3 top course asphalt;
- 100mm HL-8 base course asphalt;
- 150mm Granular 'A' Base; and,
- 450mm Granular 'B' Base.

It is anticipated the new (private) local roadways would include semi-mountable curb and gutter (OPSD 600.060), with no local sidewalks other than on the south side of the Elliott Trail West, and superelevated section with 2% cross fall throughout the condo. The profile of local roads is designed to serve as major overland flow route to the stormwater management (SWM) block. Elliott Trail West is anticipated to be assumed by Thames Centre ultimately as a public road and would have a standard crown cross section. If this arrangement does not meet with agency approval, then a private road could be considered at this location. Refer to Appendix A for Engineering Drawings for more information.

Future maintenance of all internal (condo) roadways would become the responsibility of the condominium corporation. Upon assumption of Elliott Trail West in the future, roadway maintenance would become the responsibility of Thames Centre.

5.2 Grading and Drainage Design

It is anticipated the site will require existing grades to be raised to accommodate the proposed development. Accordingly, the development area (3.2 ha) is proposed to convey drainage via new storm sewers southwest to a proposed landscaped SWM dry basin/infiltration gallery.

The proposed site has potential to restrict flow from the adjacent Nissouri Road west ditch, causing runoff to pool in the ditch system on the northeast side of the development. The design as proposed will collect this external ditch runoff through the minor system and treat / infiltrate it through the proposed condo SWMF.

There is approximately 2.73 ha of external agricultural catchment, north of the subject site which grades toward Elliott Trail West. The elevated road embankment of the Elliott Trail West extension could trap runoff in the area just north of the ROW. As such, this area and ROW drainage is proposed to be collected and bypass the private SWMF and outlet through a headwall and grassed waterway in the adjacent field to the west, also owned by the applicants, and ultimately toward the McGuffin Drain. The implementation mechanics of this ROW drainage system will be subject to further Municipal discussion but could be under section 2 of the Drainage Act. Refer to the Grading Plan (SE3) in Appendix A for more information.

5.3 Proposed Servicing

New private on-site watermains, sanitary sewers, and storm sewers are proposed to service the new development. Private condo storm sewers are proposed to an on-site stormwater management facility (SWMF) dry basin/infiltration gallery system. A new sanitary connection to Thorndale Road as well as new watermain connections to Thorndale Road and Nissouri Road are also proposed to service the development.

5.3.1 Watermain

The subject site will require a private water service for domestic and fire flow demands. A new $\pm 520\text{m}$ 150mm diameter private watermain is proposed to supply the site from a 200mm municipal watermain on Thorndale Road to the south and the Elliot Trail West extension to the north, which connects via a reducer to the 150mm diameter watermain on Nissouri Road to the east, to meet domestic demands and to supply the fire flow needs.

For private water service connections, the following Municipal design criteria are understood to apply:

- Watermains shall maintain:
 - Max Day + Fire: residual pressure not less than 140 kPa (20 psi); notwithstanding this is a private condo, velocity should not exceed 2.4 m/s.
 - Peak Hour: residential residual pressure not less than 275 kPa (40 psi); velocity may not exceed 1.5 m/s.
 - Average Day: residual pressure not less than 275 kPa (40 psi).
 - Maximum residual pressure shall not exceed 550 kPa (80 psi) and a minimum residual pressure shall not be below 275 kPa (40 psi).
 - All pressures shall be calculated/determined assuming minimum hydraulic grade line conditions apply.
- Watermains shall be designed so that water shall not remain unused in the watermain for more than three (3) days (72 hours) under average day demand.

Anticipated water demands were calculated using the Thames Centre Engineering Design standards:

- | | | |
|----------------------------|---|-----------------|
| • Average Day Demands | = | 350 L/per/day |
| • Population Density | = | 2.4 People/Unit |
| • Max Day Peaking Factor | = | 2.75 |
| • Peak Hour Peaking Factor | = | 4.13 |

A detailed water model will be completed for second submission to verify pressures, velocities, and turnover for the site.

Domestic Water Demand

Using the design population density of 2.4 persons/unit, the total design population for the site is estimated at 218 persons. Using the previously listed design parameters from the Thames Centre Engineering Design standards, the Total Average Daily Demand, Max. Day Demand, and Peak hour Demand are 53, 146, and 219 L/min, respectively. Refer to Appendix C for sanitary calculations.

Max Day + Fire

To achieve fire protection for the site, new hydrants are proposed at three (3) locations. Fire demands for select units (chosen at critical locations for the highest fire demand at each hydrant) were calculated using the Ontario Building Code (OBC) method, in accordance with Thames Centre Engineering Design standards Chapter 7.2.4 for private property.

Using the OBC method, the maximum fire demand was calculated to be 75 L/s (1,189 USGPM). Refer to SE2 in Appendix A for more information and to Appendix B for the fire flow and demand calculations.

Upon completion of the water model, colour coded hydrant collars in accordance with the requirements of NFPA 61 will be verified for the on-site hydrants in this condo development.

5.3.2 Sanitary Sewers

The development would be serviced locally through a system of 200mm diameter private sewers which would convey sewage to the existing trunk sanitary sewer on the south side Thorndale Road. Approximately 500m of local sanitary sewers are proposed to service the development.

In accordance with Thames Centre design criteria, the average per capita daily sewage production would be computed at a rate of 230 L/per/day, the extraneous inflow/infiltration allowance would be 0.10 L/s/ha, the population density would be 2.4 people/unit for all condo units, and the peak sewage flow would be derived using the Harmon Peaking Factor formula.

The proposed Draft Plan of condo concept includes 91 street townhouse units. This amounts to a design population of 219 resulting in a Harmon Peaking Factor of 3.20. The sanitary catchment area amounts to 3.1 ha as it excludes the SWM block. The peak sanitary outflow from the condo is estimated to be on the order of 2.18 L/s.

The ongoing maintenance of the private sanitary sewers would become the responsibility of the condominium corporation.

Refer to plan SE2 in the Engineering Drawing Set in Appendix A for more information and Appendix C for sanitary calculations.

5.3.3 Storm sewers and Local Drainage

The site is proposed to have a local private storm sewer system designed to collect the site runoff as well as the adjacent external runoff from the southeast neighbouring lots and direct it to an on-site dry basin/infiltration gallery SWMF.

A portion of the west roadside ditch on Nissouri Road is proposed to be regraded for ultimate ROW grading along the flanking condo site. This regrading will restrict approximately 0.19 ha of the right-of-way and ditch system from draining southerly. A ditch inlet catchbasin is proposed to collect and convey the stormwater from this ditch area to the on-site SWMF.

The proposed Elliott Trail West extension as well as approximately 2.73 ha of land north of the site is proposed to be collected and transported via a 450mm storm sewer to a headwall outlet on the Watson farm (owned by the applicant) before making its way through a grassed waterway toward the McGuffin Drain to the west. This drainage works could potentially be implemented as a mutual agreement drain under the Drainage Act with a nominal tile connection to the McGuffin Drain. Refer to SE2 in Appendix A for more information.

6.0 Stormwater Management Strategy

6.1 SWM Design Approach

The proposed scope of work includes a 91-unit condominium development as well as various parking areas, private roadways, and landscaped areas. The total impervious area will increase to approximately 59% ($C=0.62$). With no available local minor outlet for the subject site, on-site attenuation will be needed and put in place in the form of a dry-basin/infiltration/OGS system to contain, treat flows, and infiltrate up to and including the 100-year storm event. The proposed dry basin will be landscaped to provide aesthetic benefits and encourage local enjoyment as an amenity (green space) feature. Refer to SE4 in Appendix A for more information.

As noted previously, on-site surface attenuation of stormwater for the majority of the site will be employed to infiltrate post-development peak flows for all storm events up to and including the 100-year design storm event and to safely convey 250-year flows overland.

6.2 SWM Modelling

The SWM analysis was completed with PCSWMM hydrologic modelling software using the Intensity-Duration-Frequency (IDF) Parameters outlined as per the Thames Centre Design Standards. A pre-development model was created to determine post-development outflow targets and a post-development (proposed conditions) model was completed to assess the performance of the quantity control measures including the dry basin and infiltration gallery. Refer to Appendix E pre- and post-development model schematics, inputs, and outputs.

6.3 SWM Facility Design

6.3.1 SWMF Quantity Control

As noted previously, on-site surface storage and infiltration of stormwater for the site will be employed to infiltrate post-development peak flows for all storm events up to and including the 100-year design storm while safely conveying the 250-year design storm event. A private dry basin and infiltration gallery SWMF is proposed to capture the runoff from the site before recharge into the native subsoils through an infiltration gallery. A 450mm culvert in the west side of the basin would convey major overland flow (>100-year storm event) from the basin, through a proposed berm at the adjacent aggregate site, and ultimately into the adjacent site, owned by the applicants. The proposed private SWMF is 2.35m deep with approximately 3,255m³ of active storage volume in the dry basin and 300m³ of void space (750 m³ of clear stone) in the underlying infiltration gallery. The bottom of the dry basin is to be underlain with 150mm of topsoil and 500mm of clear stone for infiltration and includes three (3) inlet structures. The inlet structures are set 0.25m above the bottom of the basin to promote infiltration of the smaller events through the soil to the gravel for additional filtration. The infiltration layer is 0.50m thick by 50m long and 30m wide for a surface area of 1,500m². The gallery is to be comprised of 19mm clear stone wrapped with geotextile fabric to prevent fines from washing into the native stone subsoils.

The proposed landscaped dry basin and infiltration gallery SWMF can significantly reduce discharge rates from the subject site and external lands to below the existing runoff. Considering the existing topography of the site, and the lack of allowable outlet, this SWM strategy effectively 'over controls' the site while also providing a valuable landscape amenity to this development and achieves water balance objectives with favourable subsoils.

Table 1: Summary of Post-Development Flows and Infiltration Volumes

Design Storm Event Return Period	Pre-Development Site Peak Runoff (L/s)	Post-Development Infiltration Volume (m ³)	Post-Development Peak Overland Flow (L/s)
2-year	15	1,237	0
5-year	20	1,328	0
10-year	35	1,587	0
25-year	55	1,870	0
50-year	72	2,083	0
100-year	92	2,294	0
250-year	175	2,451	76

Table 2: Summary of SWMF Depth/Active Storage Volumes Flows

Design Storm Event Return Period	SWMF Depth ⁽¹⁾ / Elev. (m)	Peak Active Storage Volume (m ³)
2-year	1.36 / 283.11	976
5-year	1.44 / 283.19	1,062
10-year	1.65 / 283.40	1,304
25-year	1.86 / 283.61	1,569
50-year	2.01 / 283.76	1,767
100-year	2.14 / 283.89	1,967
250-year	2.34 / 284.09	2,102

Notes: (1) depth per active storage above bottom of infiltration gallery.

Table 1 above summarizes the infiltration volumes from the subject site as well as the overland flow to the lands to the west. These total outflows under the proposed ultimate condition are less than that of the peak site outflow under the existing conditions, up to and including the 250-year event. As such, it has been demonstrated that the site SWM works can provide sufficient storage and infiltration to mitigate impacts to the adjacent roadside ditch systems. Refer to Appendix D for summary SWM tables and Appendix E for the proposed conditions PCSWMM model outputs.

6.3.2 SWMF Quality Control

Multiple water quality measures are proposed within the SWM system before infiltrating into the native subsoils. The overall effect is a treatment train approach to water quality. An in-series treatment train approach is proposed as follows:

- Upstream OGS unit at the dry basin inlet, designed and sized to treat 80% of the incoming total suspended solids (TSS). See below for more information on the proposed OGS unit.
- Extended detention dry basin SWMF via no minor outlet pipe, allowing outflow to be solely based on the infiltration rates of the native subsoils. This mitigates impacts to surface waters.

- Enhanced plantings, and cable concrete armouring on the bottom of the pond inlet to reduce velocity and allow for more infiltration. These measures allow for increased settling before reaching the underlying stone gallery.
- A geotextile wrap around the infiltration gallery allows for damped velocities by extending flow path, encouraging particle settling. The geotextile wrap also provides for a level of filtration through the fabric.

Due to the sensitivity of an infiltration gallery an 'Enhanced' (80% TSS removal) level of treatment is proposed on-site through an oil grit separator (OGS) unit to pre-treat stormwater before being infiltrated. The quality control device will be installed in series with and upstream of the dry basin to provide the required level of protection. The device selected for this project is a Contech Engineering Solutions CDS (continuous deflection separation) unit Model PSMU 3035_6C or approved equivalent, for a 4.6 ha development area, with a 108 L/s peak treatment capacity based on a fine particle size distribution with an average annual TSS removal efficiency of 80.0% treating 96.1% of the average annual total runoff volume. A unit sizing has been provided by Echelon Environmental Inc. Refer to Appendix F for CDS unit cut sheets.

North of the subject site, the existing farm field and residence catchment runoff will be collected and conveyed west and the Elliott Trail West extension is proposed to be collected with the northern runoff and be directed via a 450mm outlet pipe to a 100m long grassed waterway which should allow for buffer filtration and velocity reduction. The grassed waterway is also proposed to contain two (2) rock flow check dams to promote velocity reduction to promote settling and infiltration.

6.3.3 SWMF Inlet and Outlets

Minor flows from the condo development are designed to inlet into the dry basin/infiltration gallery SWMF via a 675mm inlet pipe at 0.20%, through the CDS unit, and out a headwall. The basin below the inlet and approximately 3m of the bottom of the pond are to be lined with cable concrete mattress surrounded by turf reinforcement mat.

The dry basin is proposed to outlet stormwater via the underground infiltration gallery into the native subsoil below. The basin also includes provision for a 450mm diameter emergency overflow culvert (invert 283.90), which is only anticipated to be utilized in any storm event beyond a 100-year storm event.

6.4 Major System Conveyance

The major overland flow route for the development is proposed to convey flows to the on-site dry basin via local private roadways, which in emergency situations, will direct major flows to the lands to the west under common ownership. The onsite landscaped dry basin has been sized to store and infiltrate all major overland flows up to and including the 100-year design storm event.

6.5 Low-Impact Development

The proposed landscaped dry basin incorporates an infiltration system below the floor for storm event control and filtration. As an LID feature, the infiltration system will facilitate full, long-term recharge into native sandy subsoils after each minor precipitation event. Proposed landscape plantings within the dry basin will seek to encourage water tolerant species that have potential to support evapotranspiration and root uptake.

6.6 Water Balance

Containing and treating the site runoff before infiltrating from storm events up to and including the 100-year storm event easily exceeds pre-development targets, satisfying any water balance requirements. Refer to the Hydrogeological Report by EXP for more information on groundwater.

7.0 Erosion and Sediment Control

Complementary to the site servicing and grading design for the on-site development, erosion and sediment control (ESC) details will be included with the detailed Engineering design drawings. Temporary ESC measures will be designed to mitigate the offsite migration of sediments by incorporation of various best management practices and control measures. Typical control measures to be implemented on site include:

- Installation of silt control fencing (light duty/robust) around the site perimeter at down-gradient locations;
- Preventing silt or sediment laden runoff from entering inlets (catchbasins / catchbasin maintenance holes) by installing pre-fabricated temporary inlet filter bags and incorporating straw bale or rock dam flow checks;
- Temporary sediment traps/basins (dewatering stations where required);
- Sodding the invert of swales as soon as possible after being constructed to mitigate erosion and down cutting; in general, minimizing the duration of soil exposure in erosion prone areas by temporary vegetation coverage (i.e. hydro-seeding) is recommended;
- Maintaining sediment and erosion control structures in good repair (including periodic cleaning as required) until such time as the Engineer or the Municipality approves their removal;
- Incorporation of temporary measures at site construction entrances to minimize tracking of mud and debris onto municipal road allowances;
- Scheduling of critical conveyance works during forecasts of little to no precipitation.

An ESC plan (SE1) is provided for review in the civil drawing package.

8.0 Summary and Conclusions

The following summary outlines the main discussion points from the report:

- The 3.2 ha site is proposed as a 91-unit condominium development complete with parking, drive aisles, parks, and landscaping.
- A 150mm diameter water service connection is proposed to connect to the existing 200mm and 150mm watermains on Nissouri Road and Thorndale Road, respectively, to supply potable water and meet domestic and fire protection demands.
- Connection of a 200mm private sanitary sewer to the 300mm Thorndale Road trunk sewer is proposed to provide sewage conveyance for the proposed development.
- A private storm sewer system, complete with catchbasins and landscaped areas, is proposed to collect site runoff and convey it to the proposed on-site dry basin infiltration gallery SWMF.
- SWM quantity control is provided via a dry basin infiltration gallery SWMF to store and infiltrate all runoff from the site up to and including the 100-year design storm event.
- SWM quality control is proposed via a treatment train approach, including an oil grit separator, soil and plantings within the SWMF to filter and settle particles through velocity reduction, and a filtered stone gallery for infiltration.
- The external lands (north) and proposed Elliott Trail West extension are proposed to be contained and outlet via a 450mm storm sewer toward the McGuffin Drain via a grassed waterway complete with rock check flow dams to allow for infiltration and velocity reduction to settle particles.
- An inspection protocol, with private condo maintenance as required, should be implemented to preserve the lifespan of the storm conveyance system and quality control device.
- Erosion and sediment control measures should be implemented to help mitigate potential sediment transport and erosion resulting from construction activities.

We trust this report adequately describes the servicing strategy in support of planning approvals for this condo development project.

Development Engineering (London) Limited

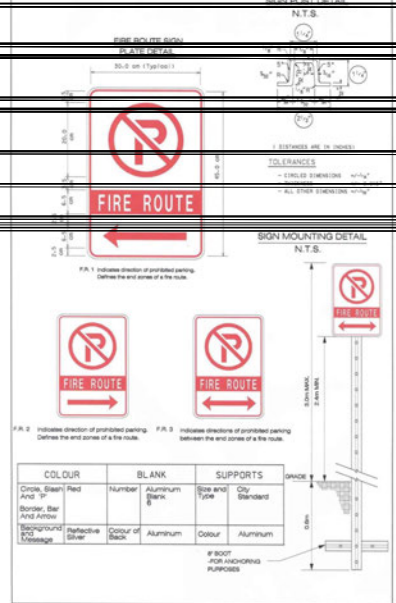
Appendix A: Engineering Drawings

PRELIMINARY
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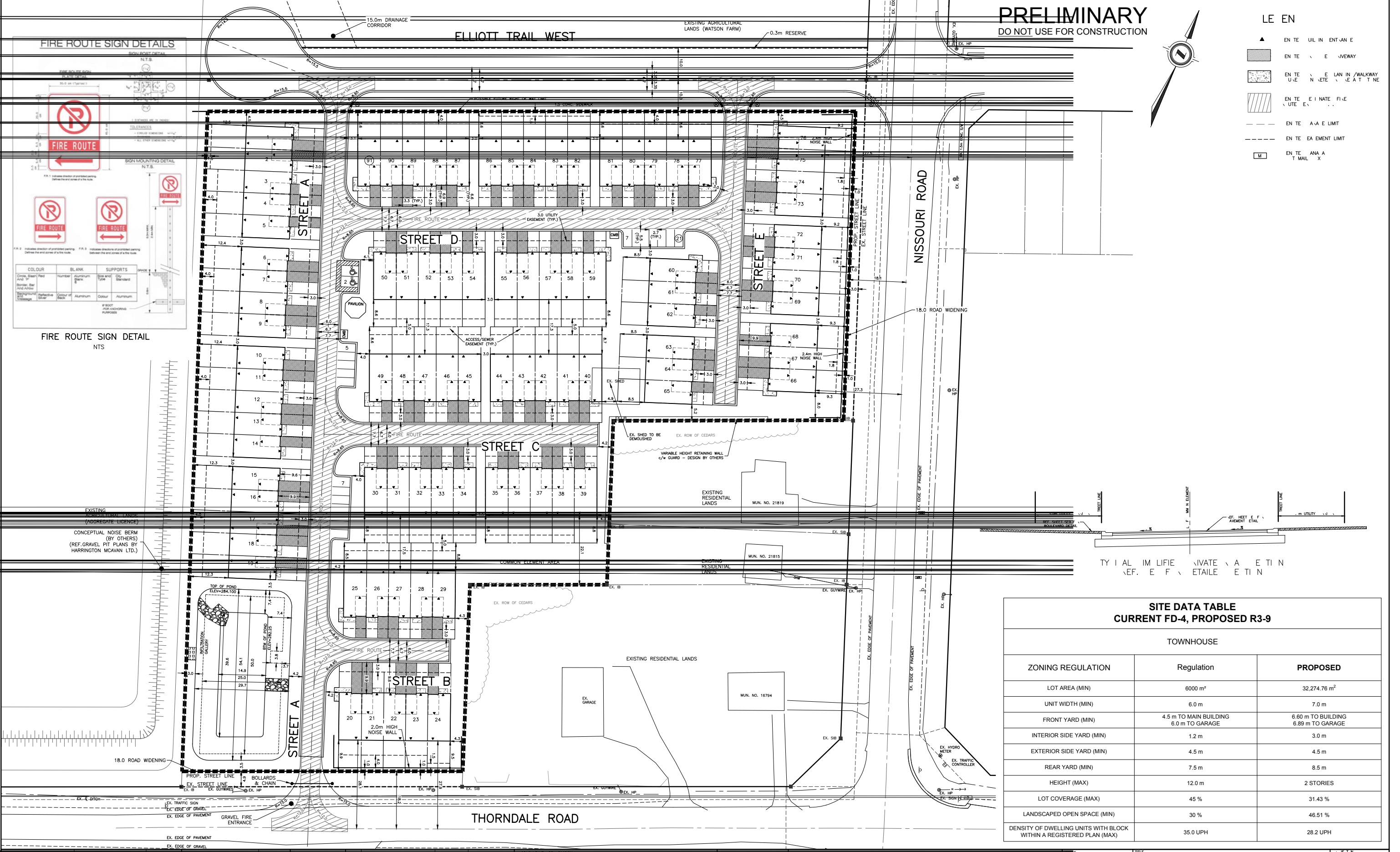


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FIRE ROUTE SIGN DETAIL NTS



SITE DATA TABLE
CURRENT FD-4, PROPOSED R3-9

ZONING REGULATION	TOWNHOUSE	
	Regulation	PROPOSED
LOT AREA (MIN)	6000 m ²	32,274.76 m ²
UNIT WIDTH (MIN)	6.0 m	7.0 m
FRONT YARD (MIN)	4.5 m TO MAIN BUILDING 6.0 m TO GARAGE	6.60 m TO BUILDING 6.89 m TO GARAGE
INTERIOR SIDE YARD (MIN)	1.2 m	3.0 m
EXTERIOR SIDE YARD (MIN)	4.5 m	4.5 m
REAR YARD (MIN)	7.5 m	8.5 m
HEIGHT (MAX)	12.0 m	2 STORIES
LOT COVERAGE (MAX)	45 %	31.43 %
LANDSCAPED OPEN SPACE (MIN)	30 %	46.51 %
DENSITY OF DWELLING UNITS WITH BLOCK WITHIN A REGISTERED PLAN (MAX)	35.0 UPH	28.2 UPH

FILE: DEL19-103 - BASE.DWG

DATE: 2023-08-08

DESIGNED BY: [Name]

CHECKED BY: [Name]

DATE: 2023-08-08

PROJECT: 41 Adelaide St. N., Unit 71

PHONE: (519) 442-1441

CONSULTING CIVIL ENGINEERS

development engineering

1757435 ONTARIO LTD.

SCALE: 1:1

TITLE: UTHEA T A T WAT N F A M A E

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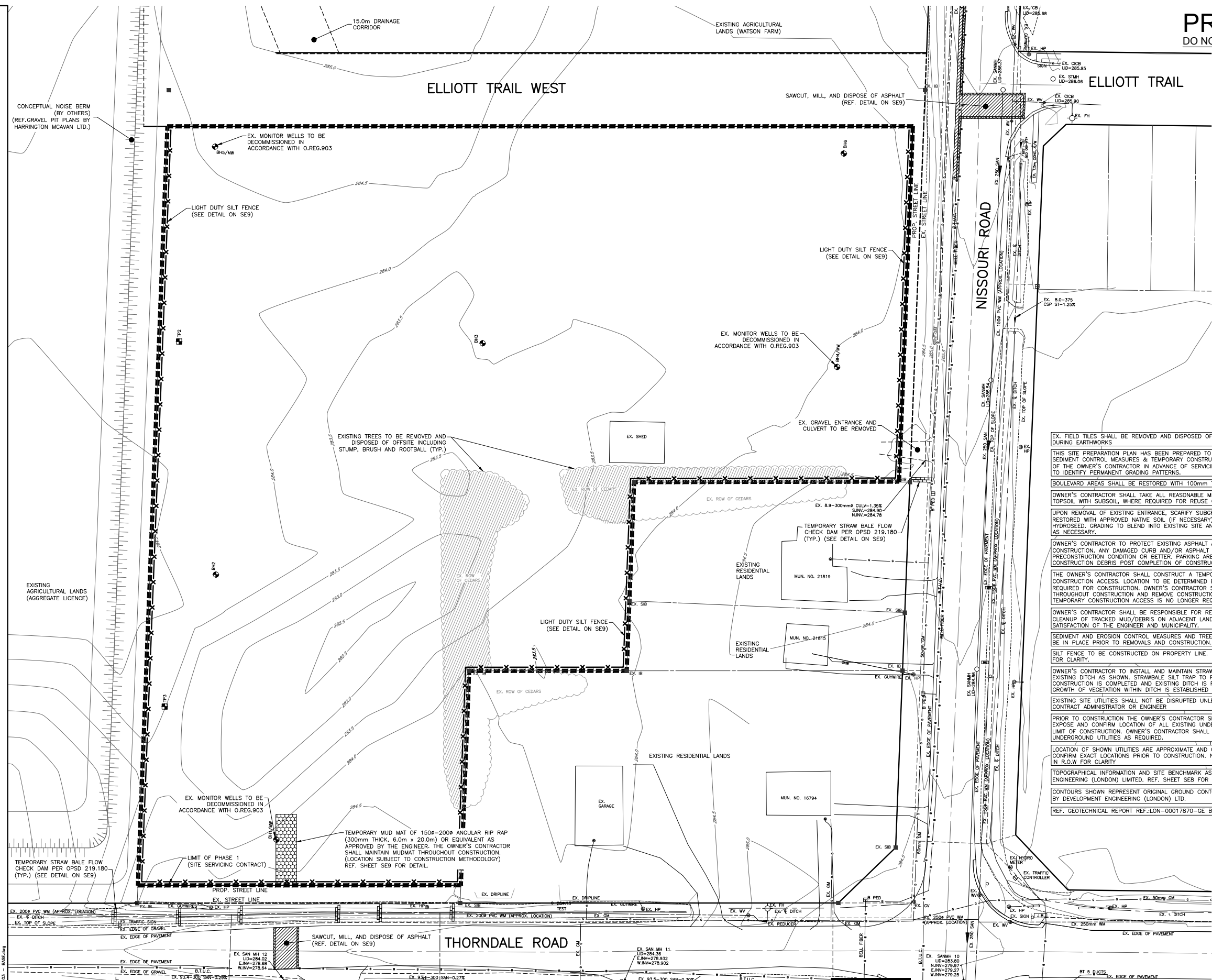
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EX. FIELD TILES SHALL BE REMOVED AND DISPOSED OF OFFSITE DURING EARTHWORKS.

THIS SITE PREPARATION PLAN HAS BEEN PREPARED TO IDENTIFY SEDIMENT CONTROL MEASURES & TEMPORARY CONSTRUCTION ACCESS OF THE OWNER'S CONTRACTOR IN ADVANCE OF SERVICING WORK TO IDENTIFY PERMANENT GRADING PATTERNS.

BOULEVARD AREAS SHALL BE RESTORED WITH 100mm TOPSOIL. OWNER'S CONTRACTOR SHALL TAKE ALL REASONABLE MEASURES TO RESTORE TOPSOIL WITH SUBSOIL, WHERE REQUIRED FOR REUSE ON SITE.

UPON REMOVAL OF EXISTING ENTRANCE, SCARIFY SUBGRADE, SITE RESTORED WITH APPROVED NATIVE SOIL (IF NECESSARY), 100mm HYDROSEED, GRADING TO BLEND INTO EXISTING SITE AND PROVIDE AS NECESSARY.

OWNER'S CONTRACTOR TO PROTECT EXISTING ASPHALT AND CURB CONSTRUCTION. ANY DAMAGED CURB AND/OR ASPHALT TO BE IN PRECONSTRUCTION CONDITION OR BETTER. PARKING AREA TO BE IN CONSTRUCTION DEBRIS POST COMPLETION OF CONSTRUCTION.

THE OWNER'S CONTRACTOR SHALL CONSTRUCT A TEMPORARY MUD CONSTRUCTION ACCESS. LOCATION TO BE DETERMINED BY OWNER REQUIRED FOR CONSTRUCTION. OWNER'S CONTRACTOR SHALL MAINTAIN THROUGHOUT CONSTRUCTION AND REMOVE CONSTRUCTION ACCESS TEMPORARY CONSTRUCTION ACCESS IS NO LONGER REQUIRED.

OWNER'S CONTRACTOR SHALL BE RESPONSIBLE FOR REGULAR CLEANUP OF TRACKED MUD/DEBRIS ON ADJACENT LANDS AND SATISFACTION OF THE ENGINEER AND MUNICIPALITY.

SEDIMENT AND EROSION CONTROL MEASURES AND TREE PRESERVATION BE IN PLACE PRIOR TO REMOVALS AND CONSTRUCTION.

SILT FENCE TO BE CONSTRUCTED ON PROPERTY LINE. LINEWORK FOR CLARITY.

OWNER'S CONTRACTOR TO INSTALL AND MAINTAIN STRAWBALE SEDIMENT CONTROL MEASURES AS SHOWN. STRAWBALE SILT TRAP TO REMAIN IN PLACE UNTIL CONSTRUCTION IS COMPLETED AND EXISTING DITCH IS RESTORED. GROWTH OF VEGETATION WITHIN DITCH IS ESTABLISHED TO MINIMUM.

EXISTING SITE UTILITIES SHALL NOT BE DISRUPTED UNLESS APPROVED BY CONTRACT ADMINISTRATOR OR ENGINEER.

PRIOR TO CONSTRUCTION THE OWNER'S CONTRACTOR SHALL EXPOSE AND CONFIRM LOCATION OF ALL EXISTING UNDERGROUND UTILITIES. OWNER'S CONTRACTOR SHALL SUPPORT UNDERGROUND UTILITIES AS REQUIRED.

LOCATION OF SHOWN UTILITIES ARE APPROXIMATE AND OWNER'S CONTRACTOR SHALL CONFIRM EXACT LOCATIONS PRIOR TO CONSTRUCTION. NOTE, N=100 IN R.O.W FOR CLARITY.

TOPOGRAPHICAL INFORMATION AND SITE BENCHMARK AS PROVIDED BY DEVELOPMENT ENGINEERING (LONDON) LIMITED. REF. SHEET SE8 FOR DATES.

CONTOURS SHOWN REPRESENT ORIGINAL GROUND CONTOURS PROVIDED BY DEVELOPMENT ENGINEERING (LONDON) LTD.

REF. GEOTECHNICAL REPORT REF: LON-00017870-GE BY EXP.

EXISTING SERVICES	DRAWING #, SOURCE	DATE	AS CONSTRUCTED SERVICES	COMPLETION	DETAILS	No.	REVISIONS	DATE	CONSULTANT
SANITARY, STORM, & WATER	STANTEC, 165500587	OCTOBER, 2010			DESIGN BY RAS	1	SPA1	JUNE 13, 2023	DEVENG
WATER	STANTEC, 165500633	FEBRUARY, 2011			DRAWN BY RAS				
					CHECKED BY SD/RAH				
					F.B.K. 1040				

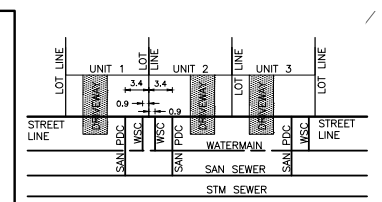
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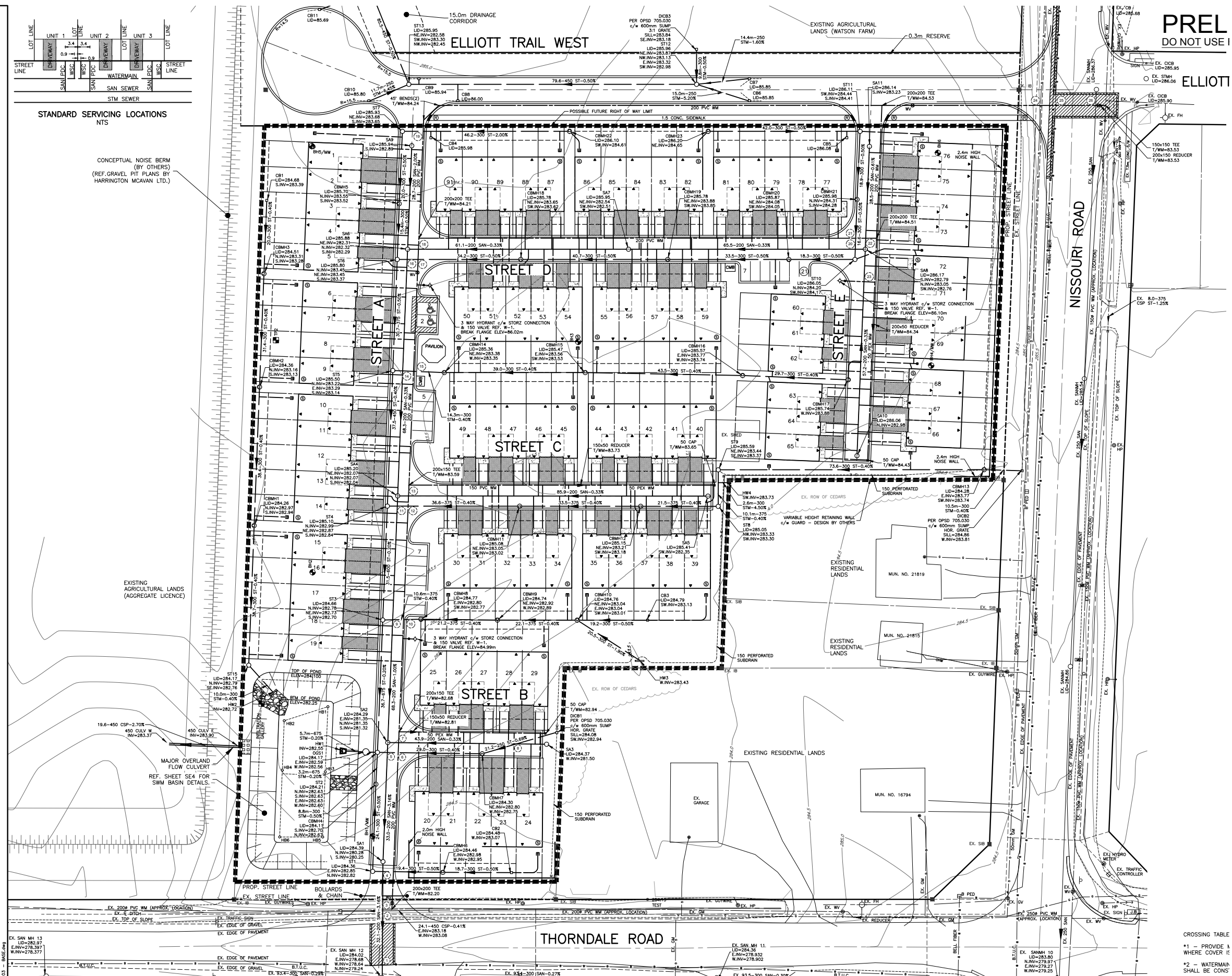
1732435 ONTARIO LTD.

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STANDARD SERVICING LOCATIONS
NTS

CONCEPTUAL NOISE BERM
(BY OTHERS)
(REF GRAVEL PIT PLANS BY
HARRINGTON MCAVAN LTD.)

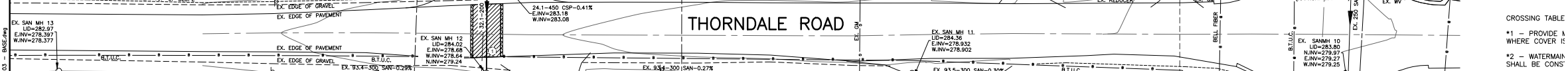


PREL
DO NOT USE

ELLIOTT

NISSOURI ROAD

THORNDALE ROAD



CROSSING TABLE
*1 - PROVIDE A WHERE COVER IS
*2 - WATERMAIN SHALL BE CONS

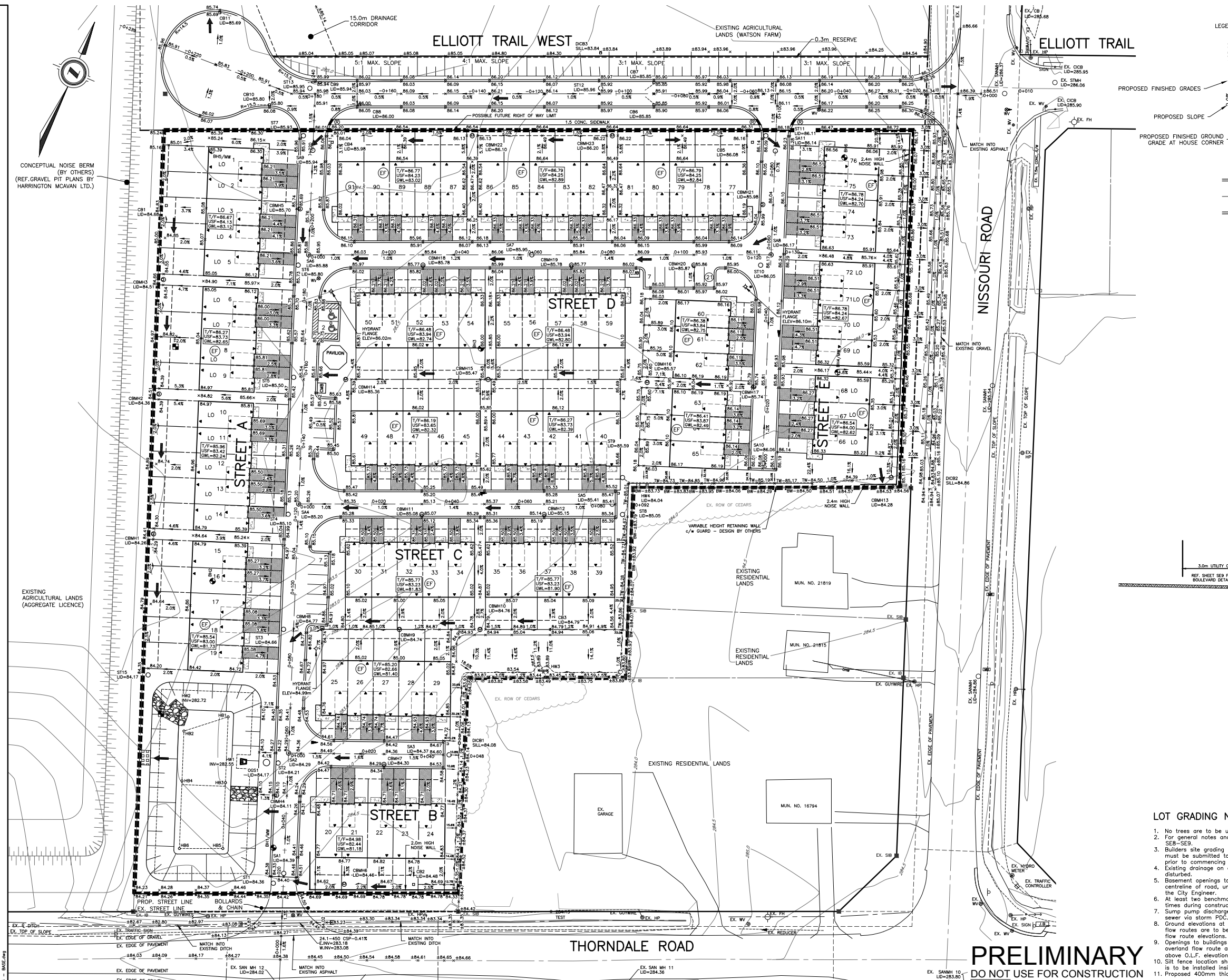
EXISTING SERVICES	DRAWING #, SOURCE	DATE	AS CONSTRUCTED SERVICES	COMPLETION	DETAILS	No.	REVISIONS	DATE	CONSULTANT	CONSULTANT OR DIVISION
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WATER	STANTEC, 165500633	FEBRUARY, 2011			DRAWN BY RAS CHECKED BY SD/RAH F.B.K. 1040					

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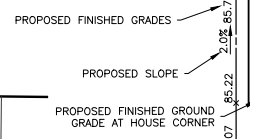
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LEGEND:



LOT GRADING NOT

1. No trees are to be unnecc
2. For general notes and det
3. Builders site grading plans must be submitted to the prior to commencing house
4. Existing drainage on abutt disturbed.
5. Basement openings to be centreline of road, unless the City Engineer.
6. At least two benchmarks 1 times during construction.
7. Sump pump discharge mu sewer via storm PDC.
8. Ground elevations at house flow routes are to be 225 flow route elevations.
9. Openings to buildings on 1 overland flow route are to be above O.L.F. elevations (1
10. Silt fence location shown is to be installed inside p
11. Proposed 400mm thick to

PRELIMINARY
DO NOT USE FOR CONSTRUCTION

EXISTING SERVICES	DRAWING #, SOURCE	DATE	AS CONSTRUCTED SERVICES	COMPLETION	DETAILS	No.	REVISIONS	DATE	CONSULTANT
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WATER	STANTEC, 165500633	FEBRUARY, 2011			DRAWN BY RAS				
					CHECKED BY SD/RAH				
					ESK 1040				

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PRIVATE SEWER DESIGNED TO O.B.C. 7.1.5.5 AND M.O.E. PIBS 6879.
PRIVATE WATERMAIN DESIGNED TO O.B.C. 7.1.5.5 AND M.O.E. PIBS 6881e.

ALL EXCESS SUBSOIL TO BE REMOVED OFFSITE BY OWNER'S CONTRACTOR AT NO EXTRA COST TO THE CONTRACTOR.

ALL SIDE SLOPES SHALL HAVE SUBGRADE FACES LIGHTLY SCARIFIED PRIOR TO PLACING 150mm ORGANIC TOPSOIL DRESSING.

EXCEPT WHERE INDICATED OTHERWISE, GRASSED AREAS ON SITE SHALL BE RESTORED WITH 150mm TOPSOIL AND HYDROSEED.

STORM SEWERS EQUAL TO OR GREATER THAN 525# SHALL BE REINFORCED CONCRETE PIPE MIN. CLASS 65D.

DRY BASIN TO BE DRESSED WITH 150mm OF TOPSOIL AND HYDROSEED WITH FLEXIBLE GROWTH MEDIUM (40% KENTUCKY BLUE GRASS, 40% CREEPING RED FESCUE, AND 20% SELF REPAIRING PERENNIAL RYEGRASS MIX.)

OIL GRIT SEPARATOR (OGS) SHALL BE CDS MODEL PMSU3035-6-C OR APPROVED EQUIVALENT. SHOP DRAWINGS ARE TO BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO INSTALLATION.

RIP RAP PROTECTION SHALL BE ANGULAR QUARRY STONE, SIZE AND DEPTH AS NOTED ON THE PLANS. AN UNDERLAY OF TERRAFIX 270R GEOTEXTILE (CLASS 2 OPSS 1860) OR APPROVED EQUIVALENT, SHALL BE PROVIDED WITH MINIMUM OVERLAPS OF 300mm IN THE DOWN GRADIENT DIRECTION AND PROPER KEY ANCHORING AT BOUNDARIES.

PRIOR TO CONSTRUCTION THE OWNER'S CONTRACTOR SHALL OBTAIN LOCATES FOR, EXPOSE AND CONFIRM LOCATION OF ALL EXISTING UNDERGROUND UTILITIES WITHIN THE LIMIT OF CONSTRUCTION. OWNER'S CONTRACTOR SHALL SUPPORT EXISTING UNDERGROUND UTILITIES AS REQUIRED.

THE OWNER'S CONSULTING ENGINEER IS REQUIRED TO INSPECT THE INSTALLATION OF SERVICES INCLUDED IN THIS PROJECT, IN ACCORDANCE WITH THE GENERAL REVIEW COMMITMENT CERTIFICATION PROCESS. THE OWNER'S CONTRACTOR IS TO ADVISE DEVELOPMENT ENGINEERING (LONDON) LTD. (519-672-8310) AT LEAST 48 HOURS PRIOR TO COMMENCING CONSTRUCTION ON THE SITE SERVICES.

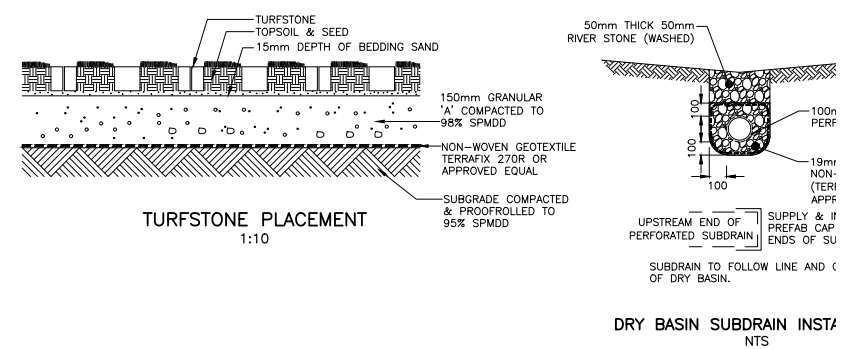
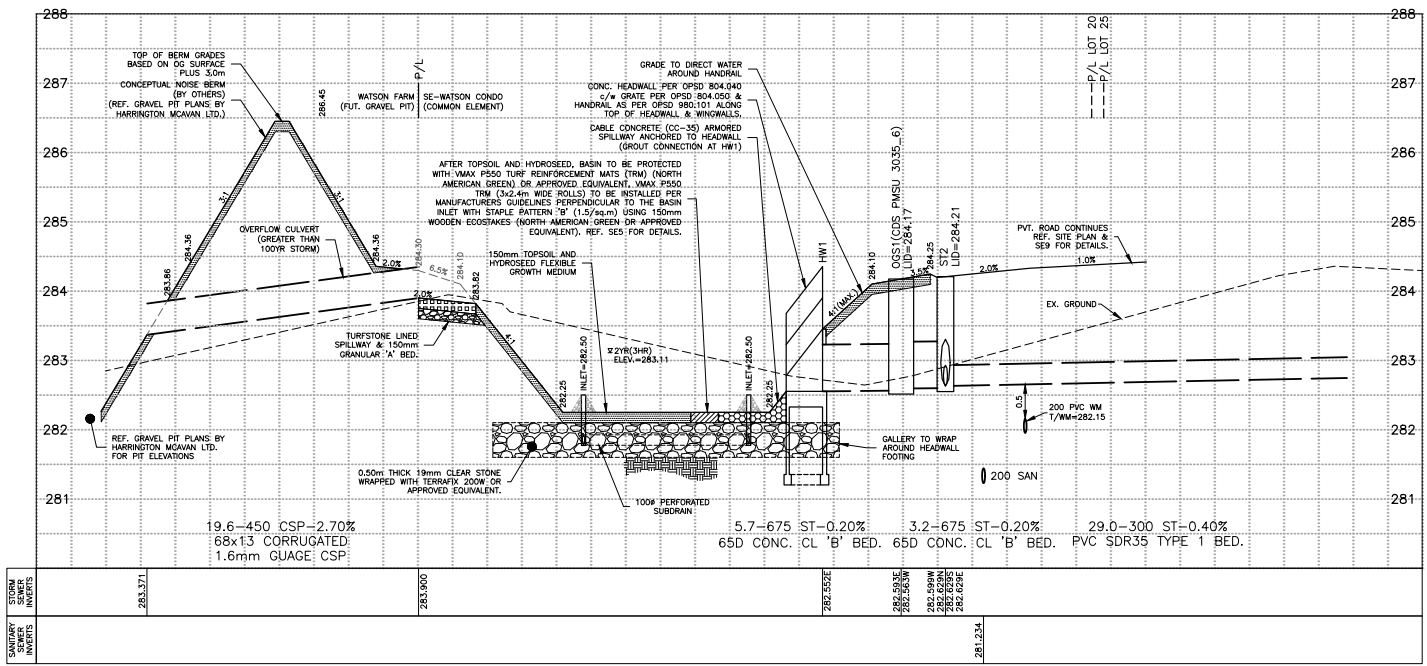
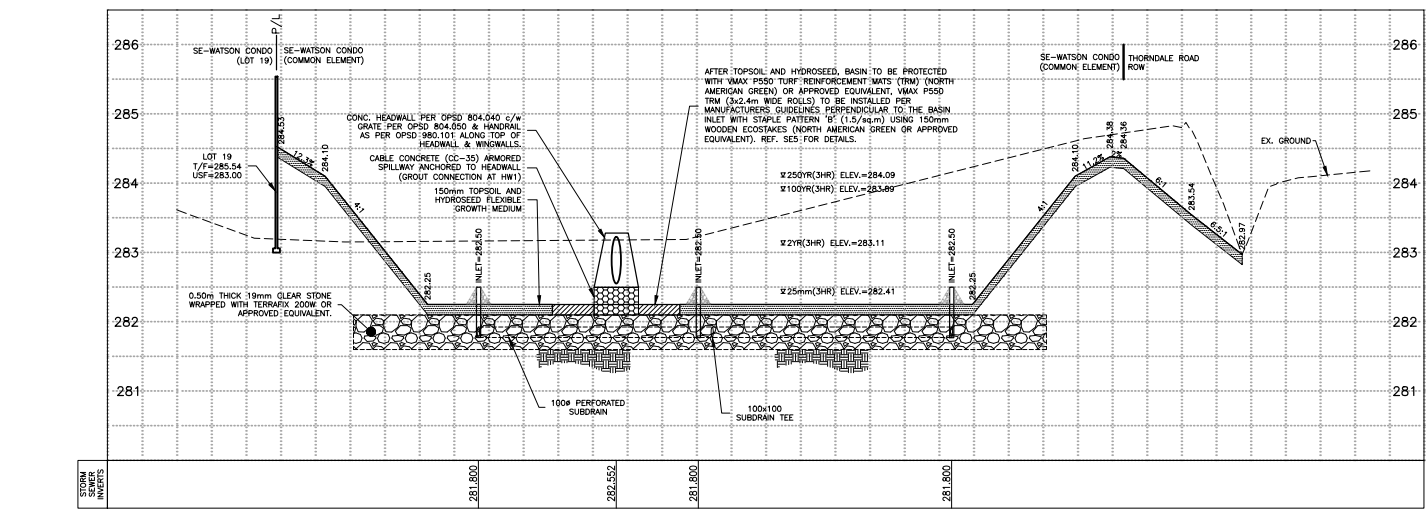
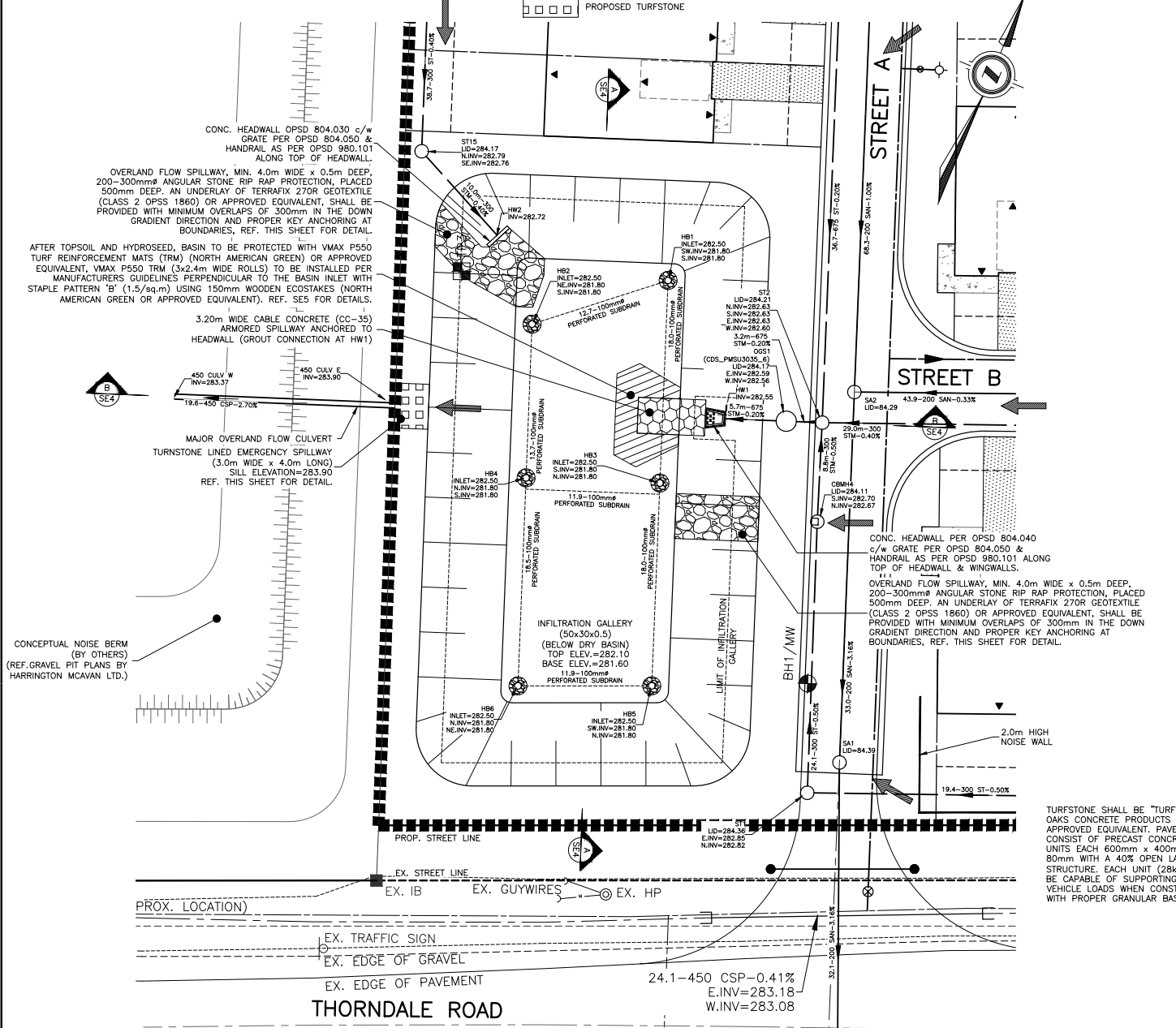
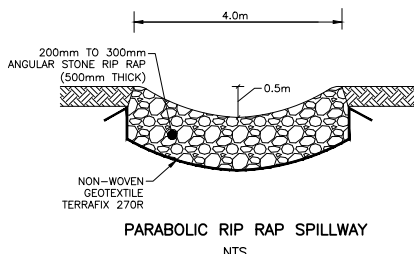
PROPOSED PEAK SITE OUTFLOW SUMMARY

STORM EVENT	MAX. HGL. ELEV. (m)
25mm	282.41
2 YEAR	283.11
5 YEAR	283.19
10 YEAR	283.40
25 YEAR	283.61
50 YEAR	283.76
100 YEAR	283.89
250 YEAR	284.09

PRIVATE DRY BASIN

LEGEND

- EX. 200 PVC WM
- EX. HP
- EX. ST1
- EX. CBMH1
- EX. CB1
- EX. HB1
- EX. ST-0.6%
- EX. SAN-0.6%
- EX. 200 PVC WM
- TP1
- EX. EXISTING GROUND CONTOUR
- EX. EXISTING GROUND GRADE
- EX. PROPOSED GRADE
- EX. EXISTING FLOW TO BE MATCHED
- OVERLAND FLOW DIRECTION
- 100mm PERFORATED SUBDRAIN
- PROPOSED BOLLARD
- PROPOSED RIP RAP ARMORING
- PROPOSED CABLE CONCRETE ARMORING (REF. DETAIL ON SHEET S5)
- PROPOSED VMX TURF REINFORCEMENT MAT
- PROPOSED TURFSTONE



PRELIMIN
DO NOT USE FOR CON

EXISTING SERVICES	DRAWING #, SOURCE	DATE	AS CONSTRUCTED SERVICES	COMPLETION	DETAILS	No.	REVISIONS	DATE	CONSULTANT
					DESIGN BY RAS	1	SPA1	JUNE 13, 2023	DEVENG
					DRAWN BY RAS				
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CDS PMSU3035-6-C DESIGN NOTES

THE STANDARD CDS PMSU3035-6-C CONFIGURATION IS SHOWN. ALTERNATE CONFIGURATIONS ARE AVAILABLE AND ARE LISTED BELOW. SOME CONFIGURATIONS MAY BE COMBINED TO SUIT SITE REQUIREMENTS.

CONFIGURATION DESCRIPTION

- GRATED INLET ONLY (NO INLET PIPE)
- GRATED INLET WITH INLET PIPE OR PIPES
- CURB INLET ONLY (NO INLET PIPE)
- CURB INLET WITH INLET PIPE OR PIPES
- CUSTOMIZABLE SUMP DEPTH AVAILABLE
- ANTI-FLOTATION DESIGN AVAILABLE UPON REQUEST

SITE SPECIFIC DATA REQUIREMENTS

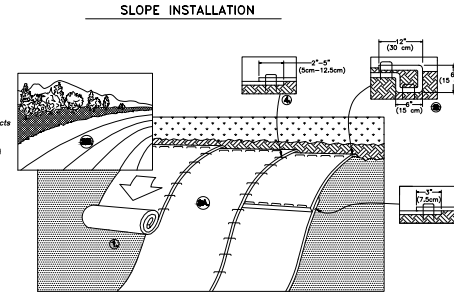
STRUCTURE ID	WATER QUALITY FLOW RATE (QFS OR LA)	PEAK FLOW RATE (QFS OR LA)	RETURN PERIOD OF PEAK FLOW (YRS)	SCREEN APERTURE (2400 OR 4700)
PIPE DATA	LEN	MATERIAL	DIAMETER	
INLET PIPE 1				
INLET PIPE 2				
OUTLET PIPE				
SDM ELEVATION				
ANTI-FLOTATION BALLAST	WIDTH	HEIGHT		
NOTES/SPECIAL REQUIREMENTS:				
PER ENGINEER OF RECORD				

GENERAL NOTES

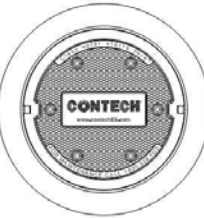
- CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
 - DIMENSIONS MARKED WITH (1) ARE REFERENCE DIMENSIONS. ACTUAL DIMENSIONS MAY VARY.
 - FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. www.contechES.com
 - CDS WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING.
 - STRUCTURE SHALL MEET AASHTO H20 AND CASTINGS SHALL MEET H20 (AASHTO M 306) LOAD RATING, ASSUMING GROUNDWATER ELEVATION AT OR BELOW THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION.
 - PVC HYDRAULIC SHEAR PLATE IS PLACED ON SHELF AT BOTTOM OF SCREEN CYLINDER. REMOVE AND REPLACE AS NECESSARY DURING MAINTENANCE CLEANING.
- INSTALLATION NOTES
- ANY SUB-BASE BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
 - CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE CDS MANHOLE STRUCTURE (LIFTING CLUTCHES PROVIDED).
 - CONTRACTOR TO ADD JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS, AND ASSEMBLE STRUCTURE.
 - CONTRACTOR TO PROVIDE, INSTALL, AND GROUT PIPES. MATCH PIPE INVERTS WITH ELEVATIONS SHOWN.
 - CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.



CDS PMSU3035-6-C
INLINE CDS
STANDARD DETAIL

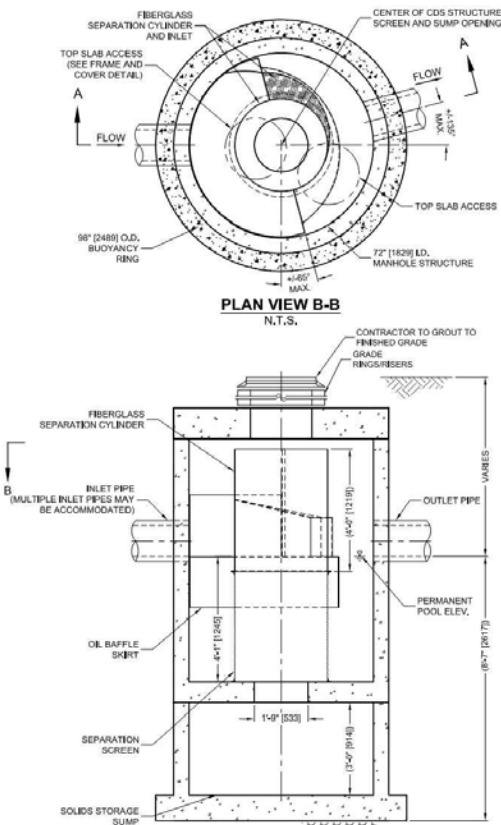


- PREPARE SOIL BEFORE INSTALLING ROLLED EROSION CONTROL PRODUCTS (RECP), INCLUDING ANY NECESSARY APPLICATION OF LIME, FERTILIZER, AND SEED.
NOTE: WHEN USING CELL-O-SEED DO NOT SEED PREPARED AREA. CELL-O-SEED MUST BE INSTALLED WITH PAPER SIDE DOWN.
- BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE RECP'S IN A 6" (15 CM) DEEP X 6" (15 CM) WIDE TRENCH WITH APPROXIMATELY 12" (30CM) OF RECP'S EXTENDED BEYOND THE UP-SLOPE PORTION OF THE TRENCH. ANCHOR THE RECP'S WITH A ROW OF STAPLES/STAKES APPROXIMATELY 12" (30 CM) APART IN THE BOTTOM OF THE TRENCH. BACKFILL AND COMPACT THE TRENCH AFTER STAPLING. APPLY SEED TO COMPACTED SOIL AND FOLD REMAINING 12" (30 CM) PORTION OF RECP'S BACK OVER SEED AND COMPACTED SOIL. SECURE RECP'S OVER COMPACTED SOIL WITH A ROW OF STAPLES/STAKES SPACED APPROXIMATELY 12" (30 CM) APART ACROSS THE WIDTH OF THE RECP'S.
- ROLL THE RECP'S (A) DOWN OR (B) HORIZONTALLY ACROSS THE SLOPE. RECP'S WILL UNROLL WITH APPROPRIATE SIDE AGAINST THE SOIL SURFACE. ALL RECP'S MUST BE SECURELY FASTENED TO SOIL SURFACE BY PLACING STAPLES/STAKES IN APPROPRIATE LOCATIONS AS SHOWN IN THE STAPLE PATTERN GUIDE. WHEN USING THE DOT SYSTEM, STAPLES/STAKES SHOULD BE PLACED THROUGH EACH OF THE COLOURED DOTS CORRESPONDING TO THE APPROPRIATE STAPLE PATTERN.
- THE EDGES OF PARALLEL RECP'S MUST BE STAPLED WITH APPROXIMATELY 2" - 5" (5 CM - 12.5 CM) OVERLAP DEPENDING ON RECP'S TYPE.
- CONSECUTIVE RECP'S SPLICED DOWN THE SLOPE MUST BE PLACED END OVER END (SHINGLE STYLE) WITH AN APPROXIMATE 3" (7.5 CM) OVERLAP. STAPLE THROUGH OVERLAPPED AREA, APPROXIMATELY 12" (30 CM) APART ACROSS ENTIRE RECP'S WIDTH.
NOTE:
IN LOOSE SOIL CONDITIONS, THE USE OF STAPLE OR STAKE LENGTHS GREATER THAN 6" (15 CM) MAY BE NECESSARY TO PROPERLY SECURE THE RECP'S.



FRAME AND COVER
(DIAMETER VARIES)
N.T.S.

PLAN VIEW B-B
N.T.S.



ELEVATION A-A
N.T.S.

PIPE DIA up to 150mm

Max. 150mm

Front face of grate 25mm dia steel rods, Typ

Max. 150mm

Fixed rods

25mm dia drill

25mm dia steel rod

25mm dia nut and lock washer

TABLE 1 - NUMBER OF RODS IN FRAME

PIPE DIA	No of RODS
450	1
525	2
600	3
675	4
750	5
825	6
900	7
975	8
1050	9
1200	10

PIPE DIA up to 1200mm

TABLE 2 - NUMBER OF FIXED RODS IN UPPER SECTION

PIPE DIA	No of RODS
1350	4
1500	5
1650	6
1800	7
1950	8
2100	9
2250	10
2400	11

PIPE DIA 1350 to 2400mm

22mm dia bolt threaded 50mm at the end to receive washer and nut, Typ

13,50mm frame

25mm dia steel rod

Cast-in hinge strap or bolt-on assembly, Typ

Mounting bracket

Hinge strap

Seal weld

ASSEMBLY

CAST-IN HINGE STRAP ASSEMBLY DETAILS

NOTES:

- Grates shall be secured by either a bolt and nut or a locking device as specified.
- Metal surfaces shall be either primed with 2 coats of self priming abrasion resistant immersion grade epoxy or hot dip galvanized as specified.
- Frame, hinge strap, mounting bracket, and steel rods shall be medium grade steel.
- All welding shall be according to CSA W59.
- All dimensions are in millimetres unless otherwise shown.

ONTARIO PROVINCIAL STANDARD DRAWING Nov 2021 Rev 3

GRATING FOR CONCRETE ENDWALL

OPSD 804.050

CONCRETE HEADWALL FOR PIPE LESS THAN 900mm DIAMETER

OPSD 804.030

Legend:
OD - Outside diameter of pipe

Notes:
A This OPSD to be read in conjunction with OPSD 3940.150.
B If a steel grate is required, refer to OPSD 804.05.
C Class of concrete: 30MPa.
D Cover to reinforcing bars 70mm ± 20mm.
E All dimensions are in millimetres unless otherwise shown.

ONTARIO PROVINCIAL STANDARD DRAWING Nov 2017 Rev 2

CONCRETE HEADWALL FOR SEWER OR CULVERT PIPE OUTLET

OPSD 804.040

Notes:
A This OPSD to be read in conjunction with OPSD 3940.150.
B Class of concrete: 30MPa.
C Cover to reinforcing bars: 75mm ± 20mm.
D Granular backfill to be placed to 300mm min thickness on all sides.
E All dimensions are in millimetres unless otherwise shown.

ONTARIO PROVINCIAL STANDARD DRAWING Nov 2017 Rev 1

BASE PLATE ELEVATION

INSTALLATION DETAIL ON EXISTING CONCRETE

ONTARIO PROVINCIAL STANDARD DRAWING Nov 2017 Rev 1

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					DRAWN BY RAS				
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CONSULTANT OR DIVISION: DEL19-103 - BASE.DWG

Drawn: Jun 13 2023 4:42 PM DEL19-103 - BASE.DWG

TOPOGRAPHICAL INFORMATION AND SITE BENCHMARK AS PROVIDED BY DEVELOPMENT ENGINEERING (LONDON) LIMITED

AREAS DISTURBED WITHIN NISSOURI ROAD SHALL BE RESTORED AS FOLLOWS:
 • 450mm OF GRAN. 'B' COMPACTED TO 98% SPMD
 • 150mm OF GRAN. 'A' COMPACTED TO 100% SPMD
 • 100mm OF HLB BINDER ASPHALT COMPACTED TO 97% MRD
 • 50mm OF HLB SHEET ASPHALT COMPACTED TO 97% MRD
 • SAWCUT ASPHALT & MILL AS PER DETAIL ON SHEET SE9
 • BOULEVARDS RESTORED WITH MIN. 150mm OF TOPSOIL AND APPROVED NURSERY SOD. (TYP.)

PRIOR TO CONSTRUCTION THE OWNER'S CONTRACTOR SHALL OBTAIN LOCATES FOR, EXPOSE AND CONFIRM LOCATION OF ALL EXISTING UNDERGROUND UTILITIES WITHIN THE LIMIT OF CONSTRUCTION. OWNER'S CONTRACTOR SHALL SUPPORT EXISTING UNDERGROUND UTILITIES AS REQUIRED TO THE SATISFACTION OF THE UTILITY OWNER

WHEN WORKING WITHIN 20m OF THE GAS MAIN, UNION GAS SHALL BE PROPERLY NOTIFIED IN ADVANCE OF ANY CONSTRUCTION ACTIVITIES

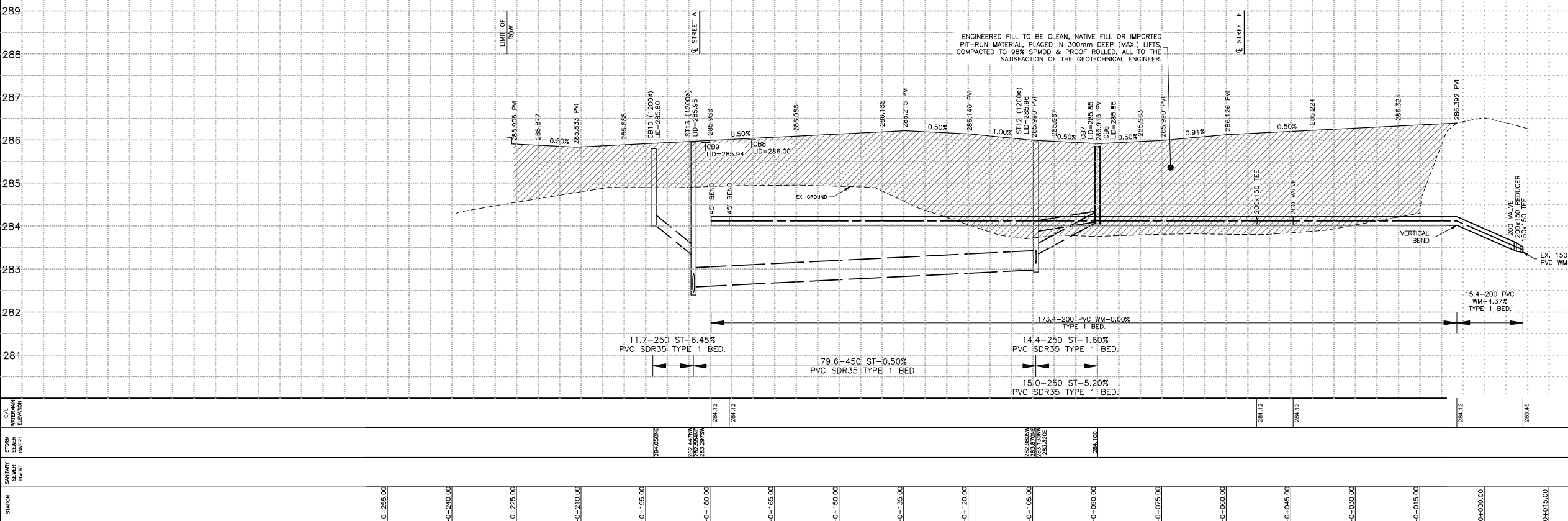
LOCATIONS OF SHOWN UTILITIES ARE APPROXIMATE AND LOCATES SHALL BE DONE BY THE OWNER'S CONTRACTOR TO CONFIRM EXACT LOCATIONS PRIOR TO CONSTRUCTION. NOTE, NOT ALL UTILITIES SHOWN IN R.O.W FOR CLARITY

REF. SHEET SE2 FOR PIPE CROSSING TABLE.

SITE BENCHMARK:

- BENCHMARK #1 - SOUTH CORNER OF TRANSFORMER PAD #504 ON NORTHEAST CORNER OF ELLIOT TRAIL & LEESBORO TRAIL.
ELEVATION=286.023m
- BENCHMARK #2 - TOP SPINDLE OF FIRE HYDRANT ON SOUTHEAST CORNER OF ELLIOT TRAIL & LEESBORO TRAIL.
ELEVATION=287.094m
- BENCHMARK #3 - HORIZONTAL SPIKE SET IN WEST FACE OF HYDRO POLE ON EAST SIDE OF NISSOURI ROAD (SECOND POLE SOUTH OF ELLIOT TRAIL).
ELEVATION=285.235m
- BENCHMARK #4 - HORIZONTAL SPIKE SET IN EAST FACE OF HYDRO POLE ON WEST SIDE OF NISSOURI ROAD AT SOUTH LOT LINE TO MN 21815.
ELEVATION=284.866m
- BENCHMARK #5 - CUT CROSS SET IN BACK OF CURB NORTHWEST CORNER OF NISSOURI ROAD & KING STREET, JUST WEST OF SPILLWAY.
ELEVATION=284.18m
- BENCHMARK #6 - TOP SPINDLE OF FIRE HYDRANT NORTH SIDE OF THORNDALE ROAD & KING STREET ±60m WEST OF NISSOURI ROAD.
ELEVATION=285.168m
- BENCHMARK #7 - HORIZONTAL SPIKE SET IN EAST FACE OF HYDRO POLE ON NORTH SIDE OF THORNDALE ROAD 3RD WEST OF NISSOURI ROAD.
ELEVATION=284.679m
- BENCHMARK #8 - HORIZONTAL SPIKE SET IN EAST FACE OF HYDRO POLE ON NORTH SIDE OF THORNDALE ROAD 5TH WEST OF NISSOURI ROAD.
ELEVATION=281.944m

CURB AND GUTTER OP&D 600.060



STATION	C/A WATERMAIN ELEVATION	STORM SEWER INVERT	SANITARY SEWER INVERT
0+255.00	284.0500	284.4200	283.7750
0+240.00	284.0500	284.4200	283.7750
0+225.00	284.0500	284.4200	283.7750
0+210.00	284.0500	284.4200	283.7750
0+195.00	284.0500	284.4200	283.7750
0+180.00	284.0500	284.4200	283.7750
0+165.00	284.0500	284.4200	283.7750
0+150.00	284.0500	284.4200	283.7750
0+135.00	284.0500	284.4200	283.7750
0+120.00	284.0500	284.4200	283.7750
0+105.00	284.0500	284.4200	283.7750
0+090.00	284.0500	284.4200	283.7750
0+075.00	284.0500	284.4200	283.7750
0+060.00	284.0500	284.4200	283.7750
0+045.00	284.0500	284.4200	283.7750
0+030.00	284.0500	284.4200	283.7750
0+015.00	284.0500	284.4200	283.7750
0+000.00	284.0500	284.4200	283.7750
0+015.00	284.0500	284.4200	283.7750

DWG: 1732435-447.dwg DATE: 2023-10-13 10:13:10

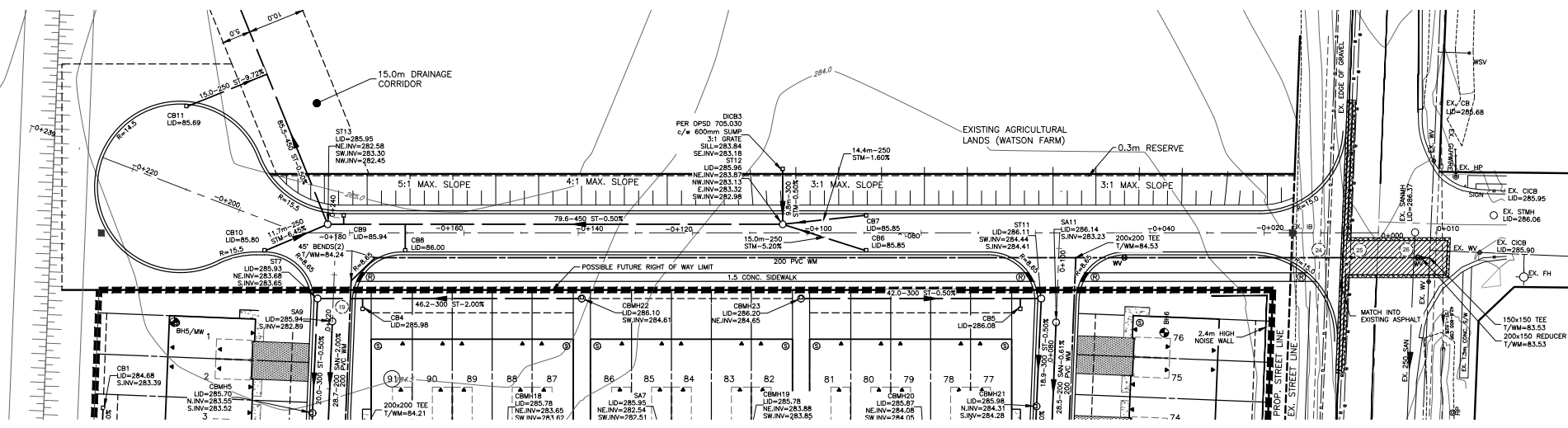
EXISTING SERVICES	DRAWING #, SOURCE	DATE	AS CONSTRUCTED SERVICES	COMPLETION	DETAILS	No.	REVISIONS	DATE	CONSULTANT
SANITARY, STORM, & WATER	STANTEC, 165500587	OCTOBER, 2010			DESIGN BY RAS DRAWN BY RAS CHECKED BY SD/RAH F.B.K. 1040	1	SPA1	JUNE 13, 2023	DEVENG

London Office
41 Adelaide St. N., Unit 71
(519) 672-8310

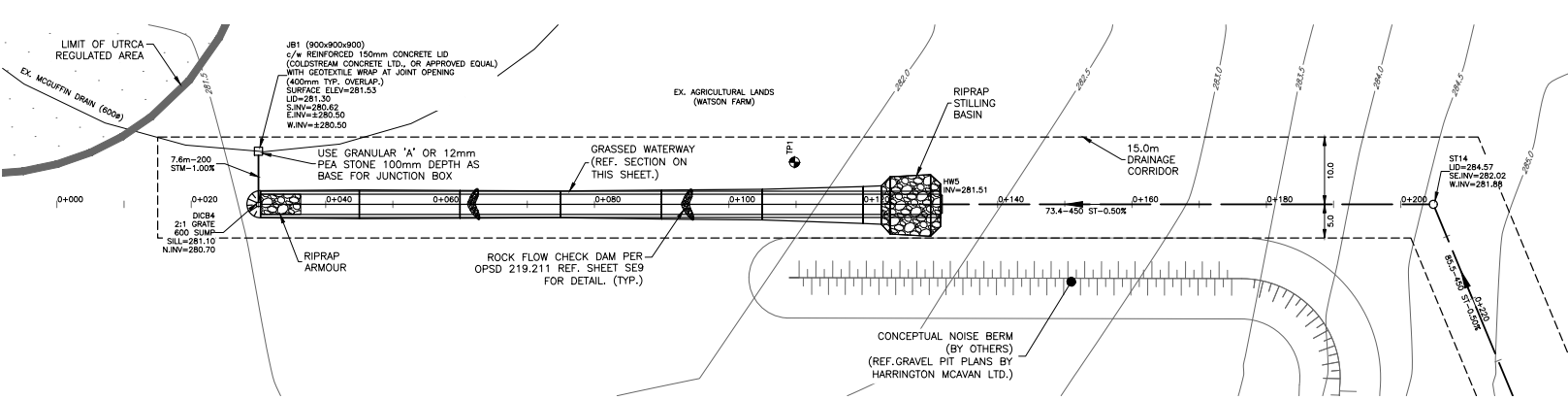
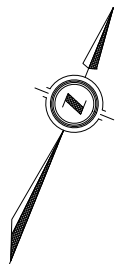
Paris Office
31 Mechanic St., Unit 301
(519) 442-1441

development engineering
(London) Limited
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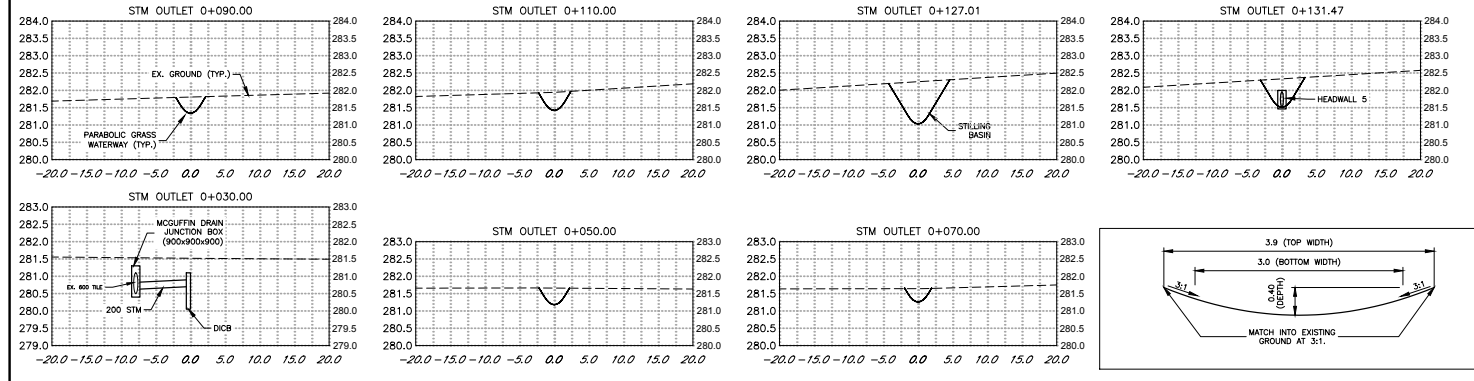
1732435 ONTARIO LTD.



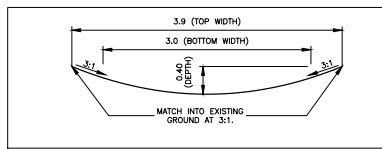
ELLIOTT TRAIL WEST



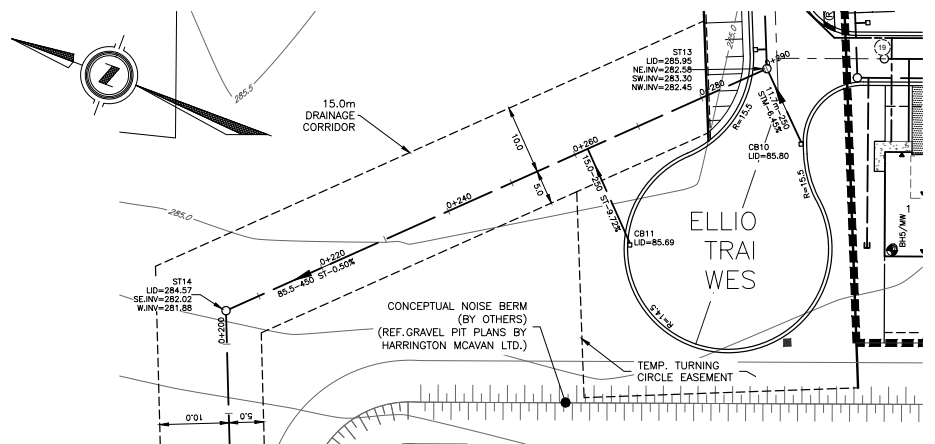
- LEGEND**
- ST1 PROPOSED STORM MANHOLE
 - SA1 PROPOSED SANITARY MANHOLE
 - ⊕ CBM1 PROPOSED CATCHBASIN MANHOLE
 - ⊕ CB1 PROPOSED STANDARD CATCHBASIN
 - ⊕ PROPOSED SUMP PUMP
 - 50.0-200 SAN-1.0% PROPOSED SANITARY SEWER
 - 50.0-600 ST-0.5% PROPOSED STORM SEWER
 - EX. 50.0-600 CSP-0.5% PROPOSED STORM CULVERT
 - 150Ø PVC WM PROPOSED WATERMAIN
 - PROPOSED WM TEE, GATE VALVE, HYDRANT WITH STORZ CONNECTION
 - FW PROPOSED VALVE
 - 150mm PERFORATED SUBDRAINAGE
 - LS DENOTES PROPOSED LIGHT SIGN
 - BH1 EXISTING BOREHOLE LOCATION (REF. GEOTECHNICAL REPORT)
 - UTRCA REGULATED AREA



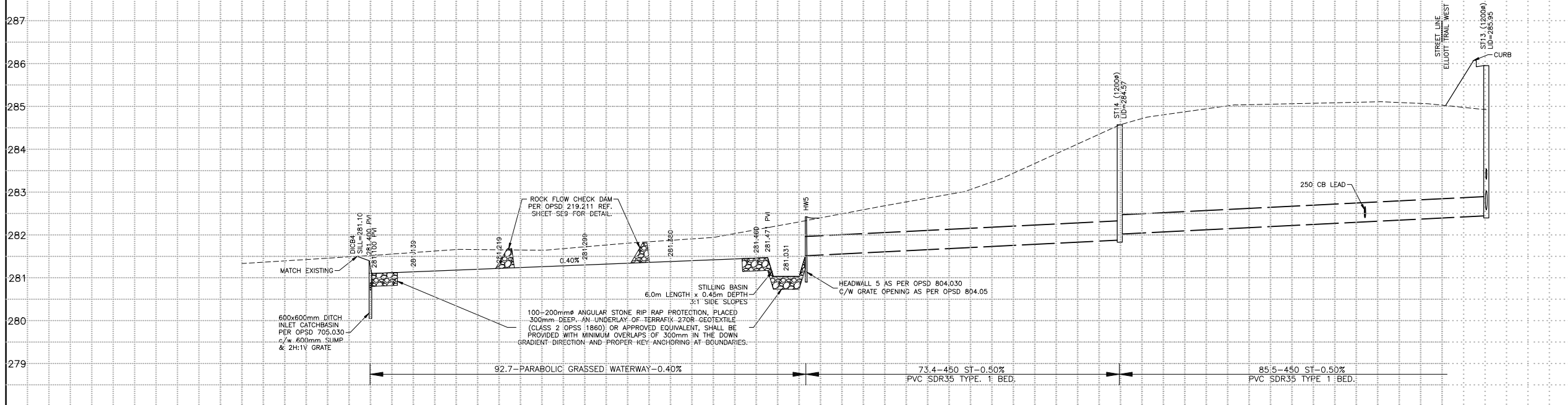
GRASSED WATERWAY SECTIONS
H=1:500, V=1:100



TYPICAL GRASSED WATERWAY SECTION
1:50



STORM OUTLET



STATION	EXISTING SERVICES	DRAWING #, SOURCE	DATE	AS CONSTRUCTED SERVICES	COMPLETION	DETAILS	No.	REVISIONS	DATE	CONSULTANT
0+000.00	MCUFFIN DRAIN	SPRIET ASSOCIATES, 78278	OCTOBER, 1979			DESIGN BY RAS DRAWN BY RAS CHECKED BY SD/RAH F.B.K. 1040	1	SPA1	JUNE 13, 2023	DEVENG

STATION	EXISTING SERVICES	DRAWING #, SOURCE	DATE	AS CONSTRUCTED SERVICES	COMPLETION	DETAILS	No.	REVISIONS	DATE	CONSULTANT
0+000.00										
0+015.00										
0+030.00										
0+045.00										
0+060.00										
0+075.00										
0+090.00										
0+105.00										
0+120.00										
0+135.00										
0+150.00										
0+165.00										
0+180.00										
0+195.00										
0+210.00										
0+225.00										
0+240.00										
0+255.00										
0+270.00										
0+285.00										
0+300.00										

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REVISED: JUN 13/23 - 447mm

FILE: DEL19-103 - BASE.DWG

Appendix B: Water Servicing Design Calculations

Watermain Demand Calculations

Determine the Domestic Water Demand:

Development Type:	Low-Density Residential	
Total Population:	91	Units
	218	Persons
Per Capita Average Daily Flow:	350.0	L/per/day
Average Daily Flow:	76,440	L/day
	53	L/min
Maximum Day Demand:	146	L/min
Maximum Hour Demand:	219*	L/min

Assumptions:

Population Density of 2.4 Persons/Unit, based on the Thames Centre Engineering Design Standards.

Maximum Day Peaking Factor = 2.75

Peak Hour Peaking Factor = 4.13

Total Average Daily Demand =	53	L/min	0.88 L/s	14 USGPM
Total Max. Day Domestic Water Demand =	146	L/min	2.43 L/s	38.57 USGPM
Total Peak Hour Domestic Water Demand =	219	L/min	3.65 L/s	57.85 USGPM

**FIRE FLOW WATERMAIN CALCULATIONS
DEL19-103 - Watson Farm Development**



Date: June 2023
By: K.Zehr, E.I.T.

Checked: R.Hern, P.Eng.

CHECK WATER FLOW DEMAND TO THE PROPOSED FIRE HYDRANT UNDER FIRE FIGHTING CONDITIONS

CALCULATE FIRE FLOW DEMAND:

Water supply for fire fighting of an unsprinklered building is determined using OBC A-3.2.5.7.

Occupancy Classification = Group C (Residential Occupancy)

Type of Construction = combustible construction. Floor assemblies are fire separations but with no fire-resistance rating. Roof assemblies, mezzanines, load bearing walls, columns and arches do not have a fire-resistance rating.

Minimum supply of water = $Q = K \cdot V \cdot S_{TOT}$

K = water supply coefficient = see Table 1 = 23

V = total volume of building in m^3 = 3168.0

S_{TOT} = spatial coefficient of all sides = $1.0 + [S_{side\ 1} + S_{side\ 2} + S_{side\ 3} + \dots \text{etc.}]$

	Separation Distance	Spatial Coeff.
N Side	>10	0.00
E Side	>10	0.00
S Side	3.0	0.50
W Side	3.0	0.50

$S_{tot} = 1.0 + [0 + 0 + 0.5 + 0.5] = 2.00$

Q = 145728.0 L, look this value up in Table 2 to determine the Required Minimum Water Supply Flow rate (L/min)

In Table 2, required minimum water supply flow rate = 4500 L/min = 75.0 L/s = 1188.8 USGPM =

Appendix C: Sanitary Design Flow Calculations

Project #: **DEL19-103**
 Project Name **Watson Farm Development**
 Date: **June 7, 2023**
 By: **K.Zehr, E.I.T.**



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PRELIMINARY SANITARY FLOW CALCULATION

91 Unit Condo Development

Population:	219 persons
Dry Weather Design Flow:	0.58 L/s
Peaking Factor (Harmon):	3.20
Total Site Infiltration Allowance:	0.31 L/s
Combined Flow:	2.18 L/s

<----(Min 2.0, Max 4.0)

Residential Site

Number of Units: **91**
 Sanitary Catchment Area: **3.1 ha**

Assumptions

Maximum Residential PPU (Medium Density): 2.40 persons/unit¹
 Residential Dry Weather Design Flow: 230 L/cap/day¹
 Peaking Factor (commercial Harmon)¹: $M_e=1.0+(14/(4+P^{0.5}))$
 Infiltration Allowance: 0.10 L/s/ha¹

References:

(1) Taken from the Thames Centre Engineering Design Standards, Dated 2021

Appendix D: Stormwater Management Summary Tables

TABLE 1: SITE CHARACTERISTICS

Runoff Area	Area (m ²)	Area (Ha)	Imp. Area (m ²)	Imperv. (%)	Runoff Coeff. (C)
Existing Conditions					
E1 - Subject Site Area	32,974	3.297	172	1%	0.20
EXT1 - Southeast Lands (Potential Future)	11,463	1.146	1,707	15%	0.30
Total Site Area =	32,974	3.297	172	1%	0.20
Total Tributary Area =	44,437	4.444	1,879	4%	0.23
Proposed Conditions					
P1 - Phase 1 & 2 - Street A, B & C	16,219	1.622	10,056	62%	0.63
P2 - Phase 3 - Street D	7,693	0.769	4,770	62%	0.63
P3 - Phase 4 - Street E	6,575	0.658	4,077	62%	0.63
P4 - Dry Basin	2,487	0.249	339	14%	0.30
P5 - Possible Future Development	12,441	1.244	7,713	62%	0.63
EXT1 - North Residential Catchment Area	27,301	2.730	1,303	5%	0.23
ROW1 - Elliot Trail West ROW	3,377	0.338	2,665	79%	0.75
ROW2 - Nissouri Road ROW	600	0.060	600	100%	0.90
Total Site Area =	45,415	4.542	26,954	59%	0.62
Total Tributary Area =	76,693	7.669	31,522	41%	0.49

TABLE 2: SUMMARY OF FLOWS

Existing Conditions = 3.297 ha
 Runoff Coefficient = 0.20 C
 % Imperviousness = 1%

Proposed Conditions = 4.542 ha
 Runoff Coefficient = 0.62 C
 % Imperviousness = 59%

Storm Event	Existing Conditions		Proposed Conditions						
	Peak Site Runoff (L/s) ⁽¹⁾	Peak Site Runoff (Including EXT1) (L/s)	Site Peak Runoff (L/s)	Peak SWMF Attenuation (m ³)	Peak SWMF Depth / Elev. (m)	Peak Runoff to McGuffin Drain (L/s)	Total Volume Infiltrated (m ³)	Peak Overflow Discharge from SWMF (L/s)	Total Peak Site Discharge (L/s)
25mm	0	2	479	398	0.66 / 282.41	87	643	0	0
2-year	15	28	909	997	1.36 / 283.11	142	1,237	0	0
5-year	20	36	1,000	1,082	1.44 / 283.19	154	1,328	0	0
10-year	35	59	1,249	1,322	1.65 / 283.40	186	1,587	0	0
25-year	55	89	1,597	1,587	1.86 / 283.61	234	1,870	0	0
50-year	72	115	1,859	1,787	2.01 / 283.76	269	2,083	0	0
100-year	92	144	2,143	1,987	2.14 / 283.89	307	2,294	0	0
250-year	175	262	2,253	2,287	2.34 / 284.09	315	2,451	76	76

1. Peak Flows Exclude EXT1



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DEL19-103: Watson Farm Lands, Thorndale

Prepared By: Kyle Zehr, E.I.T.

Checked By: Ryan Hern, P.Eng.

June 7, 2023

Assumptions

Void Ratio, Vr = **0.40**

Table 3: Infiltration Gallery Sizing and Outflow Curve

	Inlet	OUTLET	Length of Infiltration Trench* (m)	Width of Infiltration Trench (m)	Height of Infiltration Trench (m)	Trench X-Sect Area	Void Ratio	Area x Void Ratio (m2)	Storage Volume (m3)	Storage Node Area (m2)	Volume w/ Stone
DB	Dry Basin	Subsoil	50.2	30.00	0.50	15.000	0.40	6.000	301.2	602.4	753.0

* Length of pipe less 1.2m for structures.

Factored Infiltration Rate= 5.20E+01* mm/hr 1.44E-05 m/s

Storage Node	Trench/Gallery Geometry						Effective Areas		Infiltration Rates	
	Length of Infiltration Trench* (m)	Width of Infiltration Trench (m)	Height of Infiltration Trench (m)	Base Surface Area	Wall Surface Area	Porosity	Base Surface Area x Porosity	Wall Surface Area x Porosity	Base Infiltration Rate (l/s)	Base + Wall Infiltration Rate (l/s)
Infiltration Gallery	50.2	30.0	0.50	1506	80.20	1.00	1506	80.20	21.75	22.91

Outflow Curve - Infil	
Head (m)	Outflow (L/s)
0.00	21.75
0.25	22.33
0.50	22.91
3.00	22.91

Appendix E: SWM Modelling Input, Outputs, and Schematics



Legend

- ▲ Outfalls
- Subcatchments
 - < 20 %
 - 20 - 40 %
 - 40 - 60 %
 - 60 - 80 %
 - > 80 %
 - Building



50 m

DEL19-103: Watson Farm Condominium Development
Existing Conditions PCSWMM Input

```
[TITLE]
DEL19-103 - Watson Farm Lands

[OPTIONS]
;;Options      Value
;;-----
FLOW_UNITS     LPS
INFILTRATION   CURVE_NUMBER
FLOW_ROUTING   DYNWAVE
LINK_OFFSETS   ELEVATION
MIN_SLOPE      0
ALLOW_PONDING  YES
SKIP_STEADY_STATE NO

START_DATE     04/10/2015
START_TIME     00:00:00
REPORT_START_DATE 04/10/2015
REPORT_START_TIME 00:00:00
END_DATE       04/12/2015
END_TIME       00:00:00
SWEEP_START    01/01
SWEEP_END      12/31
DRY_DAYS       0
REPORT_STEP    00:01:00
WET_STEP       00:01:00
DRY_STEP       00:01:00
ROUTING_STEP   0.5

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP   0.75
LENGTHENING_STEP 0
MIN_SURFAREA    0
MAX_TRIALS      8
HEAD_TOLERANCE  0.0015
SYS_FLOW_TOL    5
LAT_FLOW_TOL    5
MINIMUM_STEP    0.5
THREADS         2

[EVAPORATION]
;;Type      Parameters
;;-----
CONSTANT    0.0
DRY_ONLY    NO

[RAINGAGES]
;;      Rain      Time      Snow      Data
;;Name   Type      Intrvl  Catch   Source
;;-----
London   INTENSITY 0:05    1.0     TIMESERIES London-25mm

[SUBCATCHMENTS]
;;
;;Name      Raingage      Outlet      Total      Pcnt.      Pcnt.      Curb      Snow
;;Name      Raingage      Outlet      Area      Imperv      Width      Slope      Length      Pack
;;-----
E1          London          OF1         3.2974    1          274.783    1.5       0
EXT1       London          OF1         1.1463    15         127.367    1         0

[SUBAREAS]
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted
;;-----
E1            0.013   0.17   2         5        25      PERVIOUS  100
EXT1         0.013   0.25   2         5        25      PERVIOUS  100

[INFILTRATION]
;;Subcatchment CurveNum HydCon DryTime
;;-----
E1            65         0.5     7
EXT1         65         0.5     7

[OUTFALLS]
;;
;;Name      Invert      Outfall      Stage/Table      Tide
;;Name      Elev.       Type         Time Series      Gate      Route To
;;-----
OF1         0           FREE         NO                 NO

[CURVES]
;;Name      Type      X-Value Y-Value
```

DEL19-103: Watson Farm Condominium Development
Existing Conditions PCSWMM Input

```
;;-----
SWMF_200828 Storage 0 2090.4
SWMF_200828 0.3 2788.9
SWMF_200828 1.75 5126.6
SWMF_200828 2.64 6056

SWMF_201005 Storage 0 2117
SWMF_201005 0.2 2721
SWMF_201005 0.4 3095
SWMF_201005 0.6 3368
SWMF_201005 0.8 3649
SWMF_201005 1 3943
SWMF_201005 1.2 4249
SWMF_201005 1.4 4578
SWMF_201005 1.6 4961
SWMF_201005 1.8 5244
SWMF_201005 2 5504
SWMF_201005 2.2 5746
SWMF_201005 2.4 5977
SWMF_201005 2.64 6088

SWMF_TEST Storage 0 2000
SWMF_TEST 2.3 4500

SWMF_v1 Storage 0 1936
SWMF_v1 2 3321
SWMF_v1 2.3 3544

SWMF_v2 Storage 0 2357
SWMF_v2 0.2 2782
SWMF_v2 0.8 3478
SWMF_v2 1.35 3891
SWMF_v2 1.65 4126

[TIMESERIES]
;;Name      Date      Time      Value
;;-----
London-100yr 0:00 5.443
London-100yr 0:05 5.819
London-100yr 0:10 6.26
London-100yr 0:15 6.783
London-100yr 0:20 7.415
London-100yr 0:25 8.196
London-100yr 0:30 9.189
London-100yr 0:35 10.496
London-100yr 0:40 12.303
London-100yr 0:45 14.978
London-100yr 0:50 19.377
London-100yr 0:55 28.048
London-100yr 1:00 53.584
London-100yr 1:05 257.109
London-100yr 1:10 126.718
London-100yr 1:15 54.547
London-100yr 1:20 34.915
London-100yr 1:25 25.869
London-100yr 1:30 20.677
London-100yr 1:35 17.306
London-100yr 1:40 14.937
London-100yr 1:45 13.178
London-100yr 1:50 11.818
London-100yr 1:55 10.734
London-100yr 2:00 9.848
London-100yr 2:05 9.109
London-100yr 2:10 8.483
London-100yr 2:15 7.946
London-100yr 2:20 7.479
London-100yr 2:25 7.07
London-100yr 2:30 6.707
London-100yr 2:35 6.383
London-100yr 2:40 6.093
London-100yr 2:45 5.831
London-100yr 2:50 5.592
London-100yr 2:55 5.375
London-100yr 3:00 0

London-10yr 0:00 3.324
London-10yr 0:05 3.596
London-10yr 0:10 3.92
London-10yr 0:15 4.312
London-10yr 0:20 4.795
```

DEL19-103: Watson Farm Condominium Development
Existing Conditions PCSWMM Input

London-10yr	0:25	5.405
London-10yr	0:30	6.2
London-10yr	0:35	7.277
London-10yr	0:40	8.814
London-10yr	0:45	11.168
London-10yr	0:50	15.184
London-10yr	0:55	23.34
London-10yr	1:00	46.748
London-10yr	1:05	164.792
London-10yr	1:10	101.694
London-10yr	1:15	47.792
London-10yr	1:20	29.832
London-10yr	1:25	21.275
London-10yr	1:30	16.389
London-10yr	1:35	13.272
London-10yr	1:40	11.129
London-10yr	1:45	9.572
London-10yr	1:50	8.395
London-10yr	1:55	7.475
London-10yr	2:00	6.738
London-10yr	2:05	6.135
London-10yr	2:10	5.633
London-10yr	2:15	5.208
London-10yr	2:20	4.844
London-10yr	2:25	4.53
London-10yr	2:30	4.255
London-10yr	2:35	4.012
London-10yr	2:40	3.797
London-10yr	2:45	3.605
London-10yr	2:50	3.432
London-10yr	2:55	3.275
London-10yr	3:00	0
London-250yr	0:00	4.785
London-250yr	0:05	5.232
London-250yr	0:10	5.77
London-250yr	0:15	6.429
London-250yr	0:20	7.252
London-250yr	0:25	8.308
London-250yr	0:30	9.705
London-250yr	0:35	11.626
London-250yr	0:40	14.412
London-250yr	0:45	18.747
London-250yr	0:50	26.224
London-250yr	0:55	41.407
London-250yr	1:00	83.174
London-250yr	1:05	254.614
London-250yr	1:10	170.473
London-250yr	1:15	85.215
London-250yr	1:20	53.354
London-250yr	1:25	37.588
London-250yr	1:30	28.475
London-250yr	1:35	22.656
London-250yr	1:40	18.672
London-250yr	1:45	15.8
London-250yr	1:50	13.647
London-250yr	1:55	11.981
London-250yr	2:00	10.66
London-250yr	2:05	9.588
London-250yr	2:10	8.705
London-250yr	2:15	7.965
London-250yr	2:20	7.337
London-250yr	2:25	6.798
London-250yr	2:30	6.332
London-250yr	2:35	5.924
London-250yr	2:40	5.565
London-250yr	2:45	5.246
London-250yr	2:50	4.961
London-250yr	2:55	4.706
London-250yr	3:00	0
London-25mm	0.083333	1.45
London-25mm	0.166667	1.54
London-25mm	0.250000	1.65
London-25mm	0.333333	1.77
London-25mm	0.416667	1.91
London-25mm	0.500000	2.09
London-25mm	0.583333	2.30
London-25mm	0.666667	2.56
London-25mm	0.750000	2.90

DEL19-103: Watson Farm Condominium Development
Existing Conditions PCSWMM Input

London-25mm	0.833333	3.37
London-25mm	0.916667	4.02
London-25mm	1.000000	5.03
London-25mm	1.083333	6.79
London-25mm	1.166667	10.64
London-25mm	1.250000	25.07
London-25mm	1.333333	77.80
London-25mm	1.416667	32.39
London-25mm	1.500000	17.77
London-25mm	1.583333	12.12
London-25mm	1.666667	9.18
London-25mm	1.750000	7.40
London-25mm	1.833333	6.21
London-25mm	1.916667	5.36
London-25mm	2.000000	4.72
London-25mm	2.083333	4.22
London-25mm	2.166667	3.83
London-25mm	2.250000	3.50
London-25mm	2.333333	3.24
London-25mm	2.416667	3.01
London-25mm	2.500000	2.81
London-25mm	2.583333	2.64
London-25mm	2.666667	2.49
London-25mm	2.750000	2.36
London-25mm	2.833333	2.24
London-25mm	2.916667	2.14
London-25mm	3.000000	2.04
London-25mm	3.083333	1.96
London-25mm	3.166667	1.88
London-25mm	3.250000	1.81
London-25mm	3.333333	1.74
London-25mm	3.416667	1.68
London-25mm	3.500000	1.62
London-25mm	3.583333	1.57
London-25mm	3.666667	1.52
London-25mm	3.750000	1.48
London-25mm	3.833333	1.43
London-25mm	3.916667	1.39
London-25mm	4.000000	1.36
London-25yr	0:00	4.21
London-25yr	0:05	4.524
London-25yr	0:10	4.894
London-25yr	0:15	5.336
London-25yr	0:20	5.876
London-25yr	0:25	6.551
London-25yr	0:30	7.417
London-25yr	0:35	8.574
London-25yr	0:40	10.199
London-25yr	0:45	12.647
London-25yr	0:50	16.757
London-25yr	0:55	25.022
London-25yr	1:00	49.335
London-25yr	1:05	202.437
London-25yr	1:10	112.952
London-25yr	1:15	50.323
London-25yr	1:20	31.618
London-25yr	1:25	22.93
London-25yr	1:30	17.982
London-25yr	1:35	14.809
London-25yr	1:40	12.608
London-25yr	1:45	10.993
London-25yr	1:50	9.759
London-25yr	1:55	8.786
London-25yr	2:00	7.998
London-25yr	2:05	7.347
London-25yr	2:10	6.8
London-25yr	2:15	6.334
London-25yr	2:20	5.932
London-25yr	2:25	5.581
London-25yr	2:30	5.272
London-25yr	2:35	4.998
London-25yr	2:40	4.754
London-25yr	2:45	4.534
London-25yr	2:50	4.335
London-25yr	2:55	4.154
London-25yr	3:00	0
London-2yr	0:00	2.649
London-2yr	0:05	2.875

DEL19-103: Watson Farm Condominium Development
Existing Conditions PCSWMM Input

```

London-2yr      0:10      3.145
London-2yr      0:15      3.472
London-2yr      0:20      3.878
London-2yr      0:25      4.393
London-2yr      0:30      5.067
London-2yr      0:35      5.984
London-2yr      0:40      7.299
London-2yr      0:45      9.322
London-2yr      0:50     12.778
London-2yr      0:55     19.764
London-2yr      1:00     39.292
London-2yr      1:05     127.195
London-2yr      1:10     82.291
London-2yr      1:15     40.209
London-2yr      1:20     25.278
London-2yr      1:25     18.005
London-2yr      1:30     13.814
London-2yr      1:35     11.133
London-2yr      1:40     9.288
London-2yr      1:45     7.95
London-2yr      1:50     6.94
London-2yr      1:55     6.153
London-2yr      2:00     5.524
London-2yr      2:05     5.011
London-2yr      2:10     4.585
London-2yr      2:15     4.226
London-2yr      2:20     3.919
London-2yr      2:25     3.655
London-2yr      2:30     3.424
London-2yr      2:35     3.221
London-2yr      2:40     3.042
London-2yr      2:45     2.882
London-2yr      2:50     2.738
London-2yr      2:55     2.608
London-2yr      3:00     0

London-50yr     0:00     4.77
London-50yr     0:05     5.114
London-50yr     0:10     5.518
London-50yr     0:15     6
London-50yr     0:20     6.585
London-50yr     0:25     7.312
London-50yr     0:30     8.242
London-50yr     0:35     9.476
London-50yr     0:40     11.198
London-50yr     0:45     13.773
London-50yr     0:50     18.06
London-50yr     0:55     26.623
London-50yr     1:00     51.93
London-50yr     1:05     229.029
London-50yr     1:10     121.172
London-50yr     1:15     52.92
London-50yr     1:20     33.445
London-50yr     1:25     24.459
London-50yr     1:30     19.334
London-50yr     1:35     16.033
London-50yr     1:40     13.732
London-50yr     1:45     12.036
London-50yr     1:50     10.734
London-50yr     1:55     9.701
London-50yr     2:00     8.862
London-50yr     2:05     8.167
London-50yr     2:10     7.58
London-50yr     2:15     7.079
London-50yr     2:20     6.644
London-50yr     2:25     6.265
London-50yr     2:30     5.93
London-50yr     2:35     5.632
London-50yr     2:40     5.365
London-50yr     2:45     5.125
London-50yr     2:50     4.907
London-50yr     2:55     4.709
London-50yr     3:00     0

London-5yr      0:00     2.846
London-5yr      0:05     3.084
London-5yr      0:10     3.368
London-5yr      0:15     3.712
London-5yr      0:20     4.137
London-5yr      0:25     4.676

```

DEL19-103: Watson Farm Condominium Development
Existing Conditions PCSWMM Input

```

London-5yr      0:30     5.38
London-5yr      0:35     6.335
London-5yr      0:40     7.702
London-5yr      0:45     9.8
London-5yr      0:50     13.382
London-5yr      0:55     20.636
London-5yr      1:00     41.127
London-5yr      1:05     137.641
London-5yr      1:10     87.426
London-5yr      1:15     42.07
London-5yr      1:20     26.379
London-5yr      1:25     18.805
London-5yr      1:30     14.457
London-5yr      1:35     11.677
London-5yr      1:40     9.765
London-5yr      1:45     8.377
London-5yr      1:50     7.329
London-5yr      1:55     6.511
London-5yr      2:00     5.857
London-5yr      2:05     5.322
London-5yr      2:10     4.877
London-5yr      2:15     4.502
London-5yr      2:20     4.181
London-5yr      2:25     3.904
London-5yr      2:30     3.662
London-5yr      2:35     3.449
London-5yr      2:40     3.26
London-5yr      2:45     3.091
London-5yr      2:50     2.94
London-5yr      2:55     2.803
London-5yr      3:00     0

[REPORT]
;;Reporting Options
INPUT      YES
CONTROLS   NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[TAGS]

[MAP]
DIMENSIONS 486991.26425 4771879.7435 487278.66675 4772230.3465
UNITS      Meters

[COORDINATES]
;;Node      X-Coord      Y-Coord
;;-----
OF1         487072.651    4771905.68

[VERTICES]
;;Link      X-Coord      Y-Coord
;;-----

[POLYGONS]
;;Subcatchment X-Coord      Y-Coord
;;-----
E1          487160.579    4771967.87
E1          487138.427    4772027.923
E1          487178.51     4772044.538
E1          487160.514    4772093.324
E1          487230.721    4772122.42
E1          487196.788    4772214.41
E1          487004.328    4772134.483
E1          487078.39     4771933.719
E1          487160.579    4771967.87
EXT1       487160.514    4772093.324
EXT1       487230.721    4772122.42
EXT1       487248.717    4772073.634
EXT1       487265.603    4772027.859
EXT1       487256.766    4772007.838
EXT1       487160.579    4771967.87
EXT1       487138.427    4772027.923
EXT1       487178.51     4772044.538
EXT1       487160.514    4772093.324

[SYMBOLS]
;;Gage      X-Coord      Y-Coord
;;-----

```


DEL19-103: Watson Farm Condominium Development
 25mm Design Storm Event – Existing Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 2
 Number of nodes 1
 Number of links 0
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-25mm	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
E1	3.30	274.78	1.00	1.5000	London	OF1
EXT1	1.15	127.37	15.00	1.0000	London	OF1

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/12/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00

Runoff Quantity Continuity	Volume hectare-m	Depth mm
*****	-----	-----
Total Precipitation	0.111	25.000
Evaporation Loss	0.000	0.000
Infiltration Loss	0.104	23.435
Surface Runoff	0.001	0.291
Final Storage	0.006	1.275

DEL19-103: Watson Farm Condominium Development
 25mm Design Storm Event – Existing Conditions PCSWMM Output

Continuity Error (%) -0.005

Flow Routing Continuity	Volume hectare-m	Volume 10^6 ltr
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.001	0.013
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.001	0.013
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
E1	25.00	0.00	0.00	23.73	0.00	0.00	0.00	0.000
EXT1	25.00	0.00	0.00	22.58	1.13	0.01	1.63	0.045

Analysis begun on: Wed Jun 07 15:44:37 2023
 Analysis ended on: Wed Jun 07 15:44:37 2023
 Total elapsed time: < 1 sec

DEL19-103: Watson Farm Condominium Development
 2-year Design Storm Event – Existing Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 2
 Number of nodes 1
 Number of links 0
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-2yr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
E1	3.30	274.78	1.00	1.5000	London	OF1
EXT1	1.15	127.37	15.00	1.0000	London	OF1

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/12/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00

Runoff Quantity Continuity	Volume hectare-m	Depth mm
*****	-----	-----
Total Precipitation	0.190	42.751
Evaporation Loss	0.000	0.000
Infiltration Loss	0.165	37.176
Surface Runoff	0.019	4.303
Final Storage	0.006	1.274

DEL19-103: Watson Farm Condominium Development
 2-year Design Storm Event – Existing Conditions PCSWMM Output

Continuity Error (%) -0.005

Flow Routing Continuity	Volume hectare-m	Volume 10^6 ltr
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.019	0.191
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.019	0.191
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
E1	42.75	0.00	0.00	38.37	3.12	0.10	15.33	0.073
EXT1	42.75	0.00	0.00	33.76	7.70	0.09	13.37	0.180

Analysis begun on: Wed Jun 07 15:45:54 2023
 Analysis ended on: Wed Jun 07 15:45:54 2023
 Total elapsed time: < 1 sec

DEL19-103: Watson Farm Condominium Development
 5-year Design Storm Event – Existing Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 2
 Number of nodes 1
 Number of links 0
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-Syr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
E1	3.30	274.78	1.00	1.5000	London	OF1
EXT1	1.15	127.37	15.00	1.0000	London	OF1

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/12/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00

Runoff Quantity Continuity	Volume hectare-m	Depth mm
Total Precipitation	0.202	45.373
Evaporation Loss	0.000	0.000
Infiltration Loss	0.172	38.802
Surface Runoff	0.024	5.311
Final Storage	0.006	1.261

DEL19-103: Watson Farm Condominium Development
 5-year Design Storm Event – Existing Conditions PCSWMM Output

Continuity Error (%) -0.006

Flow Routing Continuity	Volume hectare-m	Volume 10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.024	0.236
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.024	0.236
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
E1	45.37	0.00	0.00	40.07	4.06	0.13	19.68	0.089
EXT1	45.37	0.00	0.00	35.16	8.91	0.10	15.90	0.196

Analysis begun on: Wed Jun 07 15:46:19 2023
 Analysis ended on: Wed Jun 07 15:46:19 2023
 Total elapsed time: < 1 sec

DEL19-103: Watson Farm Condominium Development
 10-year Design Storm Event – Existing Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 2
 Number of nodes 1
 Number of links 0
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-10yr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
E1	3.30	274.78	1.00	1.5000	London	OF1
EXT1	1.15	127.37	15.00	1.0000	London	OF1

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/12/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00

Runoff Quantity Continuity	Volume hectare-m	Depth mm
Total Precipitation	0.234	52.597
Evaporation Loss	0.000	0.000
Infiltration Loss	0.191	42.938
Surface Runoff	0.037	8.407
Final Storage	0.006	1.256

DEL19-103: Watson Farm Condominium Development
 10-year Design Storm Event – Existing Conditions PCSWMM Output

Continuity Error (%) -0.007

Flow Routing Continuity	Volume hectare-m	Volume 10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.037	0.374
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.037	0.374
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
E1	52.60	0.00	0.00	44.38	6.98	0.23	34.79	0.133
EXT1	52.60	0.00	0.00	38.80	12.52	0.14	24.23	0.238

Analysis begun on: Wed Jun 07 15:46:46 2023
 Analysis ended on: Wed Jun 07 15:46:46 2023
 Total elapsed time: < 1 sec

DEL19-103: Watson Farm Condominium Development
 25-year Design Storm Event – Existing Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 2
 Number of nodes 1
 Number of links 0
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-25yr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
E1	3.30	274.78	1.00	1.5000	London	OF1
EXT1	1.15	127.37	15.00	1.0000	London	OF1

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/12/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00

Runoff Quantity Continuity	Volume hectare-m	Depth mm
*****	-----	-----
Total Precipitation	0.268	60.382
Evaporation Loss	0.000	0.000
Infiltration Loss	0.209	47.102
Surface Runoff	0.053	12.023
Final Storage	0.006	1.262

DEL19-103: Watson Farm Condominium Development
 25-year Design Storm Event – Existing Conditions PCSWMM Output

Continuity Error (%) -0.009

Flow Routing Continuity	Volume hectare-m	Volume 10^6 ltr
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.053	0.534
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.053	0.534
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
E1	60.38	0.00	0.00	48.70	10.43	0.34	54.60	0.173
EXT1	60.38	0.00	0.00	42.52	16.59	0.19	34.23	0.275

Analysis begun on: Wed Jun 07 15:47:18 2023
 Analysis ended on: Wed Jun 07 15:47:19 2023
 Total elapsed time: 00:00:01

DEL19-103: Watson Farm Condominium Development
 50-year Design Storm Event – Existing Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 2
 Number of nodes 1
 Number of links 0
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-50yr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
E1	3.30	274.78	1.00	1.5000	London	OF1
EXT1	1.15	127.37	15.00	1.0000	London	OF1

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/12/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00

Runoff Quantity Continuity	Volume hectare-m	Depth mm
Total Precipitation	0.294	66.122
Evaporation Loss	0.000	0.000
Infiltration Loss	0.222	49.912
Surface Runoff	0.066	14.948
Final Storage	0.006	1.269

DEL19-103: Watson Farm Condominium Development
 50-year Design Storm Event – Existing Conditions PCSWMM Output

Continuity Error (%) -0.010

Flow Routing Continuity	Volume hectare-m	Volume 10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.066	0.664
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.066	0.664
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
E1	66.12	0.00	0.00	51.62	13.25	0.44	72.48	0.200
EXT1	66.12	0.00	0.00	45.01	19.83	0.23	42.90	0.300

Analysis begun on: Wed Jun 07 15:47:48 2023
 Analysis ended on: Wed Jun 07 15:47:48 2023
 Total elapsed time: < 1 sec

DEL19-103: Watson Farm Condominium Development
 100-year Design Storm Event – Existing Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 2
 Number of nodes 1
 Number of links 0
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-100yr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
E1	3.30	274.78	1.00	1.5000	London	OF1
EXT1	1.15	127.37	15.00	1.0000	London	OF1

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/12/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00

Runoff Quantity	Volume hectare-m	Depth mm
Total Precipitation	0.319	71.801
Evaporation Loss	0.000	0.000
Infiltration Loss	0.234	52.560
Surface Runoff	0.080	17.972
Final Storage	0.006	1.278

DEL19-103: Watson Farm Condominium Development
 100-year Design Storm Event – Existing Conditions PCSWMM Output

Continuity Error (%) -0.012

Flow Routing Continuity	Volume hectare-m	Volume 10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.080	0.799
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.080	0.799
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
E1	71.80	0.00	0.00	54.36	16.17	0.53	92.07	0.225
EXT1	71.80	0.00	0.00	47.39	23.14	0.27	52.12	0.322

Analysis begun on: Wed Jun 07 15:48:12 2023
 Analysis ended on: Wed Jun 07 15:48:13 2023
 Total elapsed time: 00:00:01

DEL19-103: Watson Farm Condominium Development
 250-year Design Storm Event – Existing Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 2
 Number of nodes 1
 Number of links 0
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-250yr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
E1	3.30	274.78	1.00	1.5000	London	OF1
EXT1	1.15	127.37	15.00	1.0000	London	OF1

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF1	OUTFALL	0.00	0.00	0.0	

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/12/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00

Runoff Quantity	Volume hectare-m	Depth mm
Total Precipitation	0.385	86.611
Evaporation Loss	0.000	0.000
Infiltration Loss	0.258	58.055
Surface Runoff	0.121	27.296
Final Storage	0.006	1.268

DEL19-103: Watson Farm Condominium Development
 250-year Design Storm Event – Existing Conditions PCSWMM Output

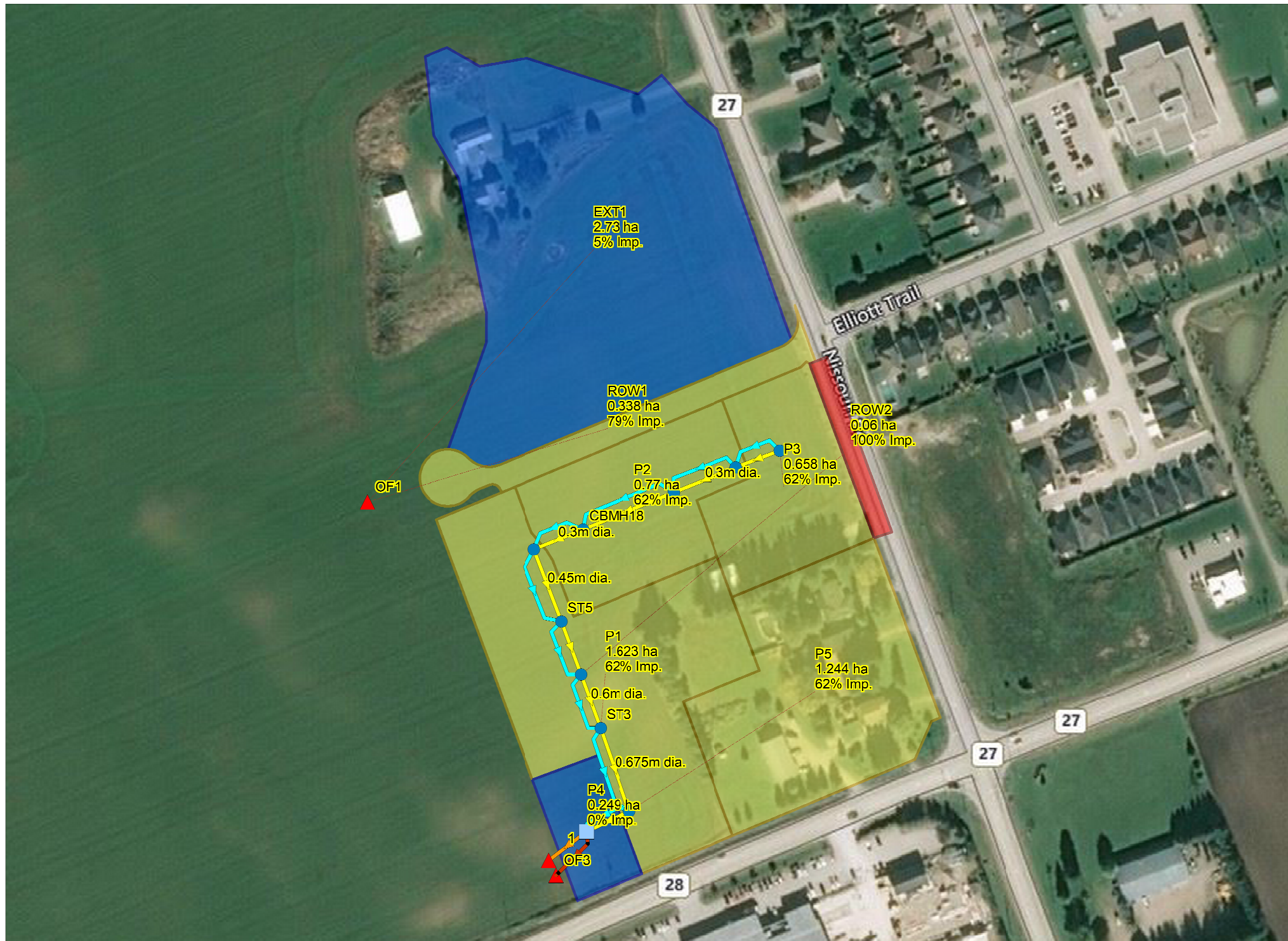
Continuity Error (%) -0.009

Flow Routing Continuity	Volume hectare-m	Volume 10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.121	1.213
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.121	1.213
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
E1	86.61	0.00	0.00	60.10	25.26	0.83	174.59	0.292
EXT1	86.61	0.00	0.00	52.18	33.16	0.38	87.30	0.383

Analysis begun on: Wed Jun 07 15:48:48 2023
 Analysis ended on: Wed Jun 07 15:48:49 2023
 Total elapsed time: 00:00:01



Legend

- Junctions
 - ▲ Outfalls
 - Storages
-
- Conduits
- Minor Sewer System
 - Major Overland Flow: Street
 - Overland Flow Routes
 - Culvert
 - Municipal Drain
 - Backflow Preventer
-
- Weirs
- Weirs
-
- Outlets
- Roof Drains
 - Inlet Control Device
 - Catchbasin Inlet
-
- Subcatchments
- < 20 %
 - 20 - 40 %
 - 40 - 60 %
 - 60 - 80 %
 - > 80 %
 - Building

DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

[TITLE]
DEL19-103 - Watson Farm Lands

[OPTIONS]
;;Options Value

FLOW_UNITS LPS
INFILTRATION CURVE_NUMBER
FLOW_ROUTING DYNWAVE
LINK_OFFSETS ELEVATION
MIN_SLOPE 0
ALLOW_PONDING YES
SKIP_STEADY_STATE NO

START_DATE 04/10/2015
START_TIME 00:00:00
REPORT_START_DATE 04/10/2015
REPORT_START_TIME 00:00:00
END_DATE 04/13/2015
END_TIME 00:00:00
SWEEP_START 01/01
SWEEP_END 12/31
DRY_DAYS 0
REPORT_STEP 00:01:00
WET_STEP 00:01:00
DRY_STEP 00:01:00
ROUTING_STEP 0.5

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP 0.75
LENGTHENING_STEP 0
MIN_SURFAREA 0
MAX_TRIALS 8
HEAD_TOLERANCE 0.0015
SYS_FLOW_TOL 5
LAT_FLOW_TOL 5
MINIMUM_STEP 0.5
THREADS 2

[EVAPORATION]
;;Type Parameters

CONSTANT 0.0
DRY_ONLY NO

[RAINGAGES]
;; Rain Time Snow Data
;;Name Type Intrvl Catch Source

London INTENSITY 0:05 1.0 TIMESERIES London-25mm

[SUBCATCHMENTS]
;; Total Pcnt. Pcnt. Curb Snow
;;Name Raingage Outlet Area Imperv Width Slope Length Pack

EXT1 London OF1 2.73 5 182 1 0
P1 London ST3 1.623 62 405.75 2 0
P2 London CBMH20 0.77 62 220 2 0
P3 London ST10 0.658 62 188 2 0
P4 London DB_INFL 0.249 0 124.5 20 0
P5 London ST2 1.244 62 311 2 0
ROW1 London OF1 0.338 79 169 2 0
ROW2 London ST4 0.06 100 24 0.2 0

[SUBAREAS]
;;Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted

EXT1 0.013 0.25 2 3 25 OUTLET
P1 0.013 0.25 2 3 25 PERVIOUS 40
P2 0.013 0.25 2 3 25 PERVIOUS 40
P3 0.013 0.25 2 3 25 PERVIOUS 40
P4 0.013 0.25 3 5 25 OUTLET
P5 0.013 0.25 2 3 25 PERVIOUS 40
ROW1 0.013 0.25 2 3 25 OUTLET
ROW2 0.013 0.25 1 2 25 OUTLET

[INFILTRATION]
;;Subcatchment CurveNum HydCon DryTime

DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

;;-----
EXT1 65 12.7 7
P1 65 12.7 7
P2 65 12.7 7
P3 65 12.7 7
P4 65 12.7 7
P5 65 12.7 7
ROW1 65 12.7 7
ROW2 65 12.7 7

[JUNCTIONS]
;; Invert Max. Init. Surcharge Ponded
;;Name Elev. Depth Depth Depth Area

CBMH18 283.61 2.53 0 0 0
CBMH19 283.84 2.31 0 0 0
CBMH20 284.04 2.14 0 0 0
ST10 284.16 2.11 0 0 0
ST2 282.6 1.86 0 0 0
ST3 282.69 2.15 0 0 0
ST4 282.83 2.53 0 0 0
ST5 283.13 2.53 0 0 0
ST6 283.36 2.6 0 0 0

[OUTFALLS]
;; Invert Outfall Stage/Table Tide
;;Name Elev. Type Time Series Gate Route To

OF1 0 FREE NO
OF2 281.75 FREE NO
OF3 283 FREE NO

[STORAGE]
;; Invert Max. Init. Storage Curve Evap.
;;Name Elev. Depth Depth Curve Params Frac.

Infiltration parameters

DB_INFL 281.75 2.35 0 TABULAR DB_230601 0 0

[CONDUITS]
;; Inlet Outlet Manning Inlet Outlet Init.
;;Max. Name Node Node Length N Offset Offset Flow

15 CBMH20 CBMH19 33.5 0.013 284.05 283.88 0 0
16 CBMH19 CBMH18 40.7 0.013 283.85 283.65 0 0
19 ST5 ST4 37.5 0.013 283.14 282.99 0 0
2 DB_INFL OF3 15 0.03 283.9 283.5 0 0
3 ST10 CBMH20 18.3 0.013 284.17 284.08 0 0
4 CBMH18 ST6 34.2 0.013 283.62 283.45 0 0
5 ST6 ST5 30.7 0.013 283.37 283.22 0 0
6 ST4 ST3 31.5 0.013 282.84 282.78 0 0
7 ST3 ST2 36.7 0.013 282.7 282.63 0 0
8 ST2 DB_INFL 15.4 0.013 282.61 282.55 0 0

[WEIRS]
;; Inlet Outlet Weir Crest Disch. Flap End End
;;Name Node Node Type Height Coeff. Gate Con. Con.
Coeff. Surcharge RoadWidth RoadSurf

10 CBMH18 ST6 TRANSVERSE 285.97 1.84 NO 0 0
YES
11 ST6 ST5 TRANSVERSE 285.8 1.84 NO 0 0
YES
12 ST4 ST3 TRANSVERSE 285.2 1.84 NO 0 0
YES
13 ST3 ST2 TRANSVERSE 284.66 1.84 NO 0 0
YES
14 ST2 DB_INFL TRANSVERSE 284.15 1.84 NO 0 0
YES
17 CBMH20 CBMH19 TRANSVERSE 286.02 1.84 NO 0 0
YES
18 CBMH19 CBMH18 TRANSVERSE 285.99 1.84 NO 0 0
YES
20 ST5 ST4 TRANSVERSE 285.5 1.84 NO 0 0
YES

DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

9 ST10 CBMH20 TRANSVERSE 286.11 1.84 NO 0 0

YES

[OUTLETS]

```
;;
Flap
;;Name
Gate
-----
1 DB_INFL OF2 281.75 TABULAR/DEPTH INFL_50x30x0.5 NO
```

[XSECTIONS]

```
;;Link
Shape Geom1 Geom2 Geom3 Geom4 Barrels
15 CIRCULAR 0.3 0 0 0 1
16 CIRCULAR 0.3 0 0 0 1
19 CIRCULAR 0.45 0 0 0 1
2 CIRCULAR 0.45 0 0 0 1
3 CIRCULAR 0.3 0 0 0 1
4 CIRCULAR 0.3 0 0 0 1
5 CIRCULAR 0.45 0 0 0 1
6 CIRCULAR 0.6 0 0 0 1
7 CIRCULAR 0.675 0 0 0 1
8 CIRCULAR 0.675 0 0 0 1
10 RECT_OPEN 0.15 15 0 0 0
11 RECT_OPEN 0.15 15 0 0 0
12 RECT_OPEN 0.15 15 0 0 0
13 RECT_OPEN 0.15 15 0 0 0
14 RECT_OPEN 0.3 5 0 0 0
17 RECT_OPEN 0.15 15 0 0 0
18 RECT_OPEN 0.15 15 0 0 0
20 RECT_OPEN 0.15 15 0 0 0
9 RECT_OPEN 0.15 15 0 0 0
```

[LOSSES]

```
;;Link
Inlet Outlet Average Flap Gate SeepageRate
-----
```

[CURVES]

```
;;Name
Type X-Value Y-Value
INFL_34_9X15X0.5 Rating 0 7.56
INFL_34_9X15X0.5 0.25 7.92
INFL_34_9X15X0.5 0.5 8.28
INFL_34_9X15X0.5 3 8.28
INFL_37X17X0.5 Rating 0 9.09
INFL_37X17X0.5 0.25 9.48
INFL_37X17X0.5 0.5 9.87
INFL_37X17X0.5 3 9.87
INFL_38_4X38_4X13 Rating 0 7.21
INFL_38_4X38_4X13 0.25 7.58
INFL_38_4X38_4X13 0.5 7.95
INFL_38_4X38_4X13 3 7.95
INFL_40_9X15X0.5 Rating 0 18.13
INFL_40_9X15X0.5 0.25 18.67
INFL_40_9X15X0.5 0.5 19.21
INFL_40_9X15X0.5 3 19.21
INFL_50x25x0.5 Rating 0 18.13
INFL_50x25x0.5 0.25 18.67
INFL_50x25x0.5 0.5 19.21
INFL_50x25x0.5 3 19.21
INFL_50x30x0.5 Rating 0 21.75
INFL_50x30x0.5 0.25 22.33
INFL_50x30x0.5 0.5 22.91
INFL_50x30x0.5 3 22.91
```

```
OptB-Gallery-Drybasin Rating 0 8.67
OptB-Gallery-Drybasin 0.1 8.83
OptB-Gallery-Drybasin 0.2 8.98
OptB-Gallery-Drybasin 0.3 9.14
OptB-Gallery-Drybasin 0.4 9.3
OptB-Gallery-Drybasin 0.5 9.46
OptB-Gallery-Drybasin 2.35 9.46
```

DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

```
DB_230601 Storage 0 602
DB_230601 0.5 602
DB_230601 0.51 585
DB_230601 2.36 1587
```

```
DrybasinOptB Storage 0 502
DrybasinOptB 0.5 502
DrybasinOptB 0.51 608
DrybasinOptB 2.35 1632
```

[TIMESERIES]

```
;;Name
Date Time Value
London-100yr 0:00 5.443
London-100yr 0:05 5.819
London-100yr 0:10 6.26
London-100yr 0:15 6.783
London-100yr 0:20 7.415
London-100yr 0:25 8.196
London-100yr 0:30 9.189
London-100yr 0:35 10.496
London-100yr 0:40 12.303
London-100yr 0:45 14.978
London-100yr 0:50 19.377
London-100yr 0:55 28.048
London-100yr 1:00 53.584
London-100yr 1:05 257.109
London-100yr 1:10 126.718
London-100yr 1:15 54.547
London-100yr 1:20 34.915
London-100yr 1:25 25.869
London-100yr 1:30 20.677
London-100yr 1:35 17.306
London-100yr 1:40 14.937
London-100yr 1:45 13.178
London-100yr 1:50 11.818
London-100yr 1:55 10.734
London-100yr 2:00 9.848
London-100yr 2:05 9.109
London-100yr 2:10 8.483
London-100yr 2:15 7.946
London-100yr 2:20 7.479
London-100yr 2:25 7.07
London-100yr 2:30 6.707
London-100yr 2:35 6.383
London-100yr 2:40 6.093
London-100yr 2:45 5.831
London-100yr 2:50 5.592
London-100yr 2:55 5.375
London-100yr 3:00 0
London-10yr 0:00 3.324
London-10yr 0:05 3.596
London-10yr 0:10 3.92
London-10yr 0:15 4.312
London-10yr 0:20 4.795
London-10yr 0:25 5.405
London-10yr 0:30 6.2
London-10yr 0:35 7.277
London-10yr 0:40 8.814
London-10yr 0:45 11.168
London-10yr 0:50 15.184
London-10yr 0:55 23.34
London-10yr 1:00 46.748
London-10yr 1:05 164.792
London-10yr 1:10 101.694
London-10yr 1:15 47.792
London-10yr 1:20 29.832
London-10yr 1:25 21.275
London-10yr 1:30 16.389
London-10yr 1:35 13.272
London-10yr 1:40 11.129
London-10yr 1:45 9.572
London-10yr 1:50 8.395
London-10yr 1:55 7.475
London-10yr 2:00 6.738
London-10yr 2:05 6.135
London-10yr 2:10 5.633
London-10yr 2:15 5.208
London-10yr 2:20 4.844
```

DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

London-10yr	2:25	4.53
London-10yr	2:30	4.255
London-10yr	2:35	4.012
London-10yr	2:40	3.797
London-10yr	2:45	3.605
London-10yr	2:50	3.432
London-10yr	2:55	3.275
London-10yr	3:00	0

London-250yr	0:00	4.785
London-250yr	0:05	5.232
London-250yr	0:10	5.77
London-250yr	0:15	6.429
London-250yr	0:20	7.252
London-250yr	0:25	8.308
London-250yr	0:30	9.705
London-250yr	0:35	11.626
London-250yr	0:40	14.412
London-250yr	0:45	18.747
London-250yr	0:50	26.224
London-250yr	0:55	41.407
London-250yr	1:00	83.174
London-250yr	1:05	254.614
London-250yr	1:10	170.473
London-250yr	1:15	85.215
London-250yr	1:20	53.354
London-250yr	1:25	37.588
London-250yr	1:30	28.475
London-250yr	1:35	22.656
London-250yr	1:40	18.672
London-250yr	1:45	15.8
London-250yr	1:50	13.647
London-250yr	1:55	11.981
London-250yr	2:00	10.66
London-250yr	2:05	9.588
London-250yr	2:10	8.705
London-250yr	2:15	7.965
London-250yr	2:20	7.337
London-250yr	2:25	6.798
London-250yr	2:30	6.332
London-250yr	2:35	5.924
London-250yr	2:40	5.565
London-250yr	2:45	5.246
London-250yr	2:50	4.961
London-250yr	2:55	4.706
London-250yr	3:00	0

London-25mm	0.083333	1.45
London-25mm	0.166667	1.54
London-25mm	0.250000	1.65
London-25mm	0.333333	1.77
London-25mm	0.416667	1.91
London-25mm	0.500000	2.09
London-25mm	0.583333	2.30
London-25mm	0.666667	2.56
London-25mm	0.750000	2.90
London-25mm	0.833333	3.37
London-25mm	0.916667	4.02
London-25mm	1.000000	5.03
London-25mm	1.083333	6.79
London-25mm	1.166667	10.64
London-25mm	1.250000	25.07
London-25mm	1.333333	77.80
London-25mm	1.416667	32.39
London-25mm	1.500000	17.77
London-25mm	1.583333	12.12
London-25mm	1.666667	9.18
London-25mm	1.750000	7.40
London-25mm	1.833333	6.21
London-25mm	1.916667	5.36
London-25mm	2.000000	4.72
London-25mm	2.083333	4.22
London-25mm	2.166667	3.83
London-25mm	2.250000	3.50
London-25mm	2.333333	3.24
London-25mm	2.416667	3.01
London-25mm	2.500000	2.81
London-25mm	2.583333	2.64
London-25mm	2.666667	2.49
London-25mm	2.750000	2.36

DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

London-25mm	2.833333	2.24
London-25mm	2.916667	2.14
London-25mm	3.000000	2.04
London-25mm	3.083333	1.96
London-25mm	3.166667	1.88
London-25mm	3.250000	1.81
London-25mm	3.333333	1.74
London-25mm	3.416667	1.68
London-25mm	3.500000	1.62
London-25mm	3.583333	1.57
London-25mm	3.666667	1.52
London-25mm	3.750000	1.48
London-25mm	3.833333	1.43
London-25mm	3.916667	1.39
London-25mm	4.000000	1.36

London-25yr	0:00	4.21
London-25yr	0:05	4.524
London-25yr	0:10	4.894
London-25yr	0:15	5.336
London-25yr	0:20	5.876
London-25yr	0:25	6.551
London-25yr	0:30	7.417
London-25yr	0:35	8.574
London-25yr	0:40	10.199
London-25yr	0:45	12.647
London-25yr	0:50	16.757
London-25yr	0:55	25.022
London-25yr	1:00	49.335
London-25yr	1:05	202.437
London-25yr	1:10	112.952
London-25yr	1:15	50.323
London-25yr	1:20	31.618
London-25yr	1:25	22.93
London-25yr	1:30	17.982
London-25yr	1:35	14.809
London-25yr	1:40	12.608
London-25yr	1:45	10.993
London-25yr	1:50	9.759
London-25yr	1:55	8.786
London-25yr	2:00	7.998
London-25yr	2:05	7.347
London-25yr	2:10	6.8
London-25yr	2:15	6.334
London-25yr	2:20	5.932
London-25yr	2:25	5.581
London-25yr	2:30	5.272
London-25yr	2:35	4.998
London-25yr	2:40	4.754
London-25yr	2:45	4.534
London-25yr	2:50	4.335
London-25yr	2:55	4.154
London-25yr	3:00	0

London-2yr	0:00	2.649
London-2yr	0:05	2.875
London-2yr	0:10	3.145
London-2yr	0:15	3.472
London-2yr	0:20	3.878
London-2yr	0:25	4.393
London-2yr	0:30	5.067
London-2yr	0:35	5.984
London-2yr	0:40	7.299
London-2yr	0:45	9.322
London-2yr	0:50	12.778
London-2yr	0:55	19.764
London-2yr	1:00	39.292
London-2yr	1:05	127.195
London-2yr	1:10	82.291
London-2yr	1:15	40.209
London-2yr	1:20	25.278
London-2yr	1:25	18.005
London-2yr	1:30	13.814
London-2yr	1:35	11.133
London-2yr	1:40	9.288
London-2yr	1:45	7.95
London-2yr	1:50	6.94
London-2yr	1:55	6.153
London-2yr	2:00	5.524
London-2yr	2:05	5.011

DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

```

London-2yr      2:10    4.585
London-2yr      2:15    4.226
London-2yr      2:20    3.919
London-2yr      2:25    3.655
London-2yr      2:30    3.424
London-2yr      2:35    3.221
London-2yr      2:40    3.042
London-2yr      2:45    2.882
London-2yr      2:50    2.738
London-2yr      2:55    2.608
London-2yr      3:00    0

London-50yr     0:00    4.77
London-50yr     0:05    5.114
London-50yr     0:10    5.518
London-50yr     0:15    6
London-50yr     0:20    6.585
London-50yr     0:25    7.312
London-50yr     0:30    8.242
London-50yr     0:35    9.476
London-50yr     0:40    11.198
London-50yr     0:45    13.773
London-50yr     0:50    18.06
London-50yr     0:55    26.623
London-50yr     1:00    51.93
London-50yr     1:05    229.029
London-50yr     1:10    121.172
London-50yr     1:15    52.92
London-50yr     1:20    33.445
London-50yr     1:25    24.459
London-50yr     1:30    19.334
London-50yr     1:35    16.033
London-50yr     1:40    13.732
London-50yr     1:45    12.036
London-50yr     1:50    10.734
London-50yr     1:55    9.701
London-50yr     2:00    8.862
London-50yr     2:05    8.167
London-50yr     2:10    7.58
London-50yr     2:15    7.079
London-50yr     2:20    6.644
London-50yr     2:25    6.265
London-50yr     2:30    5.93
London-50yr     2:35    5.632
London-50yr     2:40    5.365
London-50yr     2:45    5.125
London-50yr     2:50    4.907
London-50yr     2:55    4.709
London-50yr     3:00    0

London-5yr      0:00    2.846
London-5yr      0:05    3.084
London-5yr      0:10    3.368
London-5yr      0:15    3.712
London-5yr      0:20    4.137
London-5yr      0:25    4.676
London-5yr      0:30    5.38
London-5yr      0:35    6.335
London-5yr      0:40    7.702
London-5yr      0:45    9.8
London-5yr      0:50    13.382
London-5yr      0:55    20.636
London-5yr      1:00    41.127
London-5yr      1:05    137.641
London-5yr      1:10    87.426
London-5yr      1:15    42.07
London-5yr      1:20    26.379
London-5yr      1:25    18.805
London-5yr      1:30    14.457
London-5yr      1:35    11.677
London-5yr      1:40    9.765
London-5yr      1:45    8.377
London-5yr      1:50    7.329
London-5yr      1:55    6.511
London-5yr      2:00    5.857
London-5yr      2:05    5.322
London-5yr      2:10    4.877
London-5yr      2:15    4.502
London-5yr      2:20    4.181
London-5yr      2:25    3.904

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DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

```

London-5yr      2:30    3.662
London-5yr      2:35    3.449
London-5yr      2:40    3.26
London-5yr      2:45    3.091
London-5yr      2:50    2.94
London-5yr      2:55    2.803
London-5yr      3:00    0

[REPORT]
;;Reporting Options
INPUT      YES
CONTROLS   NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[TAGS]
Node      DB_INFL      Surface
Link      2              Culvert
Link      1              IC

[MAP]
DIMENSIONS 486952.7494 4771911.25575 487285.2706 4772405.44725
UNITS      Meters

[COORDINATES]
;;Node      X-Coord      Y-Coord
;;-----
CBMH18     487081.655   4772129.197
CBMH19     487129.713   4772148.531
CBMH20     487162.546   4772161.735
ST10       487185.592   4772170.52
ST2        487105.875   4771980.833
ST3        487090.734   4772024.689
ST4        487080.514   4772052.938
ST5        487070.253   4772080.83
ST6        487055.767   4772118.823
OF1        486967.864   4772143.827
OF2        487063.321   4771954.976
OF3        487067.189   4771947.063
DB_INFL    487083.157   4771970.161

[VERTICES]
;;Link      X-Coord      Y-Coord
;;-----
2          487083.894   4771963.745
10         487073.154   4772132.911
10         487059.259   4772127.239
11         487050.845   4772109.612
11         487061.391   4772082.661
12         487076.023   4772047.328
12         487084.14    4772024.703
13         487086.915   4772018.756
13         487087.838   4772016.039
13         487097.93    4771986.135
14         487100.598   4771982.941
14         487083.394   4771974.302
17         487157.696   4772166.955
17         487129.574   4772156.19
18         487122.323   4772153.333
18         487083.215   4772137.075
20         487064.907   4772075.337
20         487073.548   4772052.927
9          487180.242   4772175.855
9          487165.097   4772169.86

[POLYGONS]
;;Subcatchment X-Coord      Y-Coord
;;-----
EXT1       487190.197   4772233.329
EXT1       487152.882   4772340.009
EXT1       487137.097   4772352.893
EXT1       487123.811   4772368.285
EXT1       487111.411   4772358.274
EXT1       487052.531   4772377.325
EXT1       487027.62    4772372.987
EXT1       487010.446   4772382.984
EXT1       486998.557   4772378.268
EXT1       487003.472   4772336.707
EXT1       487016.494   4772313.883

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DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

EXT1	487030.822	4772243.124
EXT1	487030.822	4772227.798
EXT1	487010.639	4772171.528
EXT1	487011.724	4772171.471
EXT1	487012.801	4772171.336
EXT1	487013.867	4772171.123
EXT1	487014.914	4772170.834
EXT1	487015.937	4772170.47
EXT1	487016.931	4772170.032
EXT1	487017.891	4772169.524
EXT1	487018.812	4772168.947
EXT1	487019.688	4772168.306
EXT1	487020.516	4772167.603
EXT1	487021.291	4772166.841
EXT1	487021.291	4772166.841
EXT1	487022.05	4772166.107
EXT1	487022.859	4772165.428
EXT1	487023.714	4772164.808
EXT1	487024.611	4772164.249
EXT1	487025.545	4772163.755
EXT1	487026.511	4772163.328
EXT1	487027.504	4772162.97
EXT1	487028.521	4772162.683
EXT1	487029.555	4772162.467
EXT1	487030.602	4772162.326
EXT1	487031.656	4772162.258
EXT1	487032.712	4772162.265
EXT1	487033.765	4772162.345
EXT1	487034.81	4772162.5
EXT1	487035.842	4772162.728
EXT1	487036.854	4772163.028
EXT1	487037.844	4772163.399
EXT1	487037.844	4772163.399
EXT1	487186.484	4772225.128
EXT1	487186.763	4772225.248
EXT1	487187.04	4772225.373
EXT1	487187.315	4772225.504
EXT1	487187.586	4772225.641
EXT1	487187.854	4772225.783
EXT1	487188.12	4772225.931
EXT1	487188.382	4772226.085
EXT1	487188.641	4772226.244
EXT1	487188.897	4772226.408
EXT1	487189.149	4772226.578
EXT1	487189.397	4772226.753
EXT1	487189.642	4772226.933
EXT1	487189.883	4772227.119
EXT1	487190.12	4772227.309
EXT1	487190.353	4772227.504
EXT1	487190.581	4772227.704
EXT1	487190.806	4772227.909
EXT1	487191.026	4772228.119
EXT1	487191.242	4772228.333
EXT1	487191.453	4772228.551
EXT1	487191.66	4772228.774
EXT1	487191.862	4772229.001
EXT1	487191.862	4772229.001
EXT1	487190.197	4772233.329
P1	487062.595	4772118.108
P1	487054.506	4772127.613
P1	487045.643	4772151.64
P1	487004.328	4772134.483
P1	487054.865	4771997.487
P1	487089.579	4772010.293
P1	487112.585	4771947.928
P1	487160.579	4771967.87
P1	487134.521	4772038.511
P1	487174.756	4772054.716
P1	487160.514	4772093.324
P1	487152.556	4772114.897
P1	487076.673	4772083.415
P1	487065.799	4772112.892
P1	487062.595	4772118.108
P2	487141.659	4772144.439
P2	487170.531	4772156.429
P2	487155.473	4772197.252
P2	487045.643	4772151.64
P2	487054.506	4772127.613
P2	487062.595	4772118.108
P2	487065.799	4772112.892

DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

P2	487076.673	4772083.415
P2	487152.556	4772114.897
P2	487141.659	4772144.439
P3	487199.671	4772217.232
P3	487201.43	4772216.272
P3	487235.246	4772124.435
P3	487230.721	4772122.42
P3	487160.514	4772093.324
P3	487152.556	4772114.897
P3	487141.659	4772144.439
P3	487170.531	4772156.429
P3	487155.473	4772197.252
P3	487196.788	4772214.41
P3	487199.671	4772217.232
P4	487054.865	4771997.487
P4	487089.579	4772010.293
P4	487112.585	4771947.928
P4	487078.39	4771933.719
P4	487054.865	4771997.487
P5	487160.579	4771967.87
P5	487134.521	4772038.511
P5	487174.756	4772054.716
P5	487160.514	4772093.324
P5	487230.721	4772122.42
P5	487235.246	4772124.435
P5	487270.156	4772029.803
P5	487265.598	4772027.872
P5	487256.766	4772007.838
P5	487160.579	4771967.87
ROW1	487038.526	4772148.685
ROW1	487038.017	4772150.065
ROW1	487037.897	4772150.356
ROW1	487037.758	4772150.639
ROW1	487037.599	4772150.911
ROW1	487037.422	4772151.171
ROW1	487037.226	4772151.419
ROW1	487037.014	4772151.652
ROW1	487036.786	4772151.869
ROW1	487036.543	4772152.07
ROW1	487036.287	4772152.254
ROW1	487036.019	4772152.419
ROW1	487035.74	4772152.565
ROW1	487035.451	4772152.691
ROW1	487035.154	4772152.797
ROW1	487034.851	4772152.881
ROW1	487034.542	4772152.945
ROW1	487034.229	4772152.986
ROW1	487033.915	4772153.005
ROW1	487033.6	4772153.003
ROW1	487033.286	4772152.978
ROW1	487032.974	4772152.932
ROW1	487032.666	4772152.863
ROW1	487032.364	4772152.774
ROW1	487032.069	4772152.663
ROW1	487032.069	4772152.663
ROW1	487027.236	4772150.656
ROW1	487026.961	4772150.538
ROW1	487026.689	4772150.416
ROW1	487026.42	4772150.288
ROW1	487026.153	4772150.155
ROW1	487025.889	4772150.016
ROW1	487025.627	4772149.872
ROW1	487025.369	4772149.723
ROW1	487025.114	4772149.569
ROW1	487024.861	4772149.41
ROW1	487024.612	4772149.246
ROW1	487024.366	4772149.077
ROW1	487024.124	4772148.903
ROW1	487023.885	4772148.724
ROW1	487023.65	4772148.541
ROW1	487023.418	4772148.353
ROW1	487023.191	4772148.16
ROW1	487022.967	4772147.963
ROW1	487022.747	4772147.762
ROW1	487022.531	4772147.556
ROW1	487022.319	4772147.346
ROW1	487022.111	4772147.132
ROW1	487021.908	4772146.913
ROW1	487021.908	4772146.913
ROW1	487021.207	4772146.132

DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

ROW1	487020.454	4772145.401
ROW1	487019.651	4772144.725
ROW1	487018.803	4772144.107
ROW1	487017.913	4772143.549
ROW1	487016.987	4772143.056
ROW1	487016.029	4772142.628
ROW1	487015.043	4772142.268
ROW1	487014.034	4772141.978
ROW1	487013.007	4772141.759
ROW1	487011.968	4772141.613
ROW1	487010.921	4772141.539
ROW1	487009.871	4772141.539
ROW1	487008.824	4772141.613
ROW1	487007.785	4772141.759
ROW1	487006.758	4772141.978
ROW1	487005.749	4772142.268
ROW1	487004.763	4772142.628
ROW1	487003.805	4772143.056
ROW1	487002.879	4772143.55
ROW1	487001.99	4772144.108
ROW1	487001.141	4772144.726
ROW1	487000.339	4772145.402
ROW1	486999.585	4772146.133
ROW1	486998.884	4772146.914
ROW1	486998.24	4772147.743
ROW1	486997.655	4772148.615
ROW1	486997.133	4772149.525
ROW1	486996.675	4772150.47
ROW1	486996.285	4772151.444
ROW1	486995.964	4772152.443
ROW1	486995.714	4772153.463
ROW1	486995.535	4772154.497
ROW1	486995.429	4772155.541
ROW1	486995.397	4772156.591
ROW1	486995.438	4772157.639
ROW1	486995.552	4772158.683
ROW1	486995.739	4772159.716
ROW1	486995.997	4772160.733
ROW1	486996.327	4772161.73
ROW1	486996.725	4772162.701
ROW1	486997.19	4772163.642
ROW1	486997.719	4772164.548
ROW1	486998.311	4772165.415
ROW1	486998.962	4772166.238
ROW1	486999.669	4772167.014
ROW1	487000.428	4772167.739
ROW1	487001.237	4772168.408
ROW1	487002.09	4772169.02
ROW1	487002.983	4772169.57
ROW1	487003.914	4772170.057
ROW1	487004.875	4772170.477
ROW1	487005.864	4772170.829
ROW1	487006.875	4772171.111
ROW1	487007.904	4772171.322
ROW1	487008.944	4772171.46
ROW1	487009.992	4772171.525
ROW1	487011.041	4772171.516
ROW1	487012.088	4772171.435
ROW1	487013.126	4772171.28
ROW1	487014.151	4772171.053
ROW1	487015.157	4772170.755
ROW1	487016.14	4772170.387
ROW1	487017.095	4772169.951
ROW1	487018.017	4772169.45
ROW1	487018.902	4772168.885
ROW1	487019.746	4772168.26
ROW1	487020.543	4772167.578
ROW1	487021.291	4772166.841
ROW1	487021.291	4772166.841
ROW1	487022.05	4772166.107
ROW1	487022.859	4772165.428
ROW1	487023.714	4772164.808
ROW1	487024.611	4772164.249
ROW1	487025.545	4772163.755
ROW1	487026.511	4772163.328
ROW1	487027.504	4772162.97
ROW1	487028.521	4772162.683
ROW1	487029.555	4772162.467
ROW1	487030.602	4772162.326
ROW1	487031.656	4772162.258

DEL19-103: Watson Farm Condominium Development
Proposed Conditions PCSWMM Input

ROW1	487032.712	4772162.265
ROW1	487033.765	4772162.345
ROW1	487034.81	4772162.5
ROW1	487035.842	4772162.728
ROW1	487036.854	4772163.028
ROW1	487037.844	4772163.399
ROW1	487037.844	4772163.399
ROW1	487186.484	4772225.128
ROW1	487187.403	4772225.548
ROW1	487188.29	4772226.03
ROW1	487189.141	4772226.573
ROW1	487189.953	4772227.174
ROW1	487190.72	4772227.83
ROW1	487191.441	4772228.538
ROW1	487192.11	4772229.294
ROW1	487192.725	4772230.095
ROW1	487193.282	4772230.937
ROW1	487193.78	4772231.816
ROW1	487194.216	4772232.727
ROW1	487194.587	4772233.666
ROW1	487194.891	4772234.629
ROW1	487195.128	4772235.611
ROW1	487195.296	4772236.606
ROW1	487195.394	4772237.611
ROW1	487195.422	4772238.621
ROW1	487195.38	4772239.63
ROW1	487195.268	4772240.633
ROW1	487195.086	4772241.627
ROW1	487194.835	4772242.605
ROW1	487194.517	4772243.563
ROW1	487194.517	4772243.563
ROW1	487193.843	4772246.931
ROW1	487209.609	4772203.937
ROW1	487207.246	4772209.018
ROW1	487206.854	4772209.984
ROW1	487206.393	4772210.919
ROW1	487205.867	4772211.819
ROW1	487205.278	4772212.679
ROW1	487204.628	4772213.494
ROW1	487203.921	4772214.261
ROW1	487203.161	4772214.974
ROW1	487202.352	4772215.631
ROW1	487201.497	4772216.229
ROW1	487200.602	4772216.763
ROW1	487199.671	4772217.232
ROW1	487199.671	4772217.232
ROW1	487196.788	4772214.41
ROW1	487155.473	4772197.252
ROW1	487045.643	4772151.64
ROW1	487038.526	4772148.685
ROW2	487235.246	4772124.435
ROW2	487201.497	4772216.229
ROW2	487210.726	4772219.76
ROW2	487216.927	4772203.619
ROW2	487221.53	4772191.175
ROW2	487226.738	4772176.728
ROW2	487232.012	4772162.206
ROW2	487237.261	4772148.084
ROW2	487242.588	4772133.505
ROW2	487244.776	4772127.51
ROW2	487235.246	4772124.435

[SYMBOLS]
;;Gage X-Coord Y-Coord
;;-----

DEL19-103: Watson Farm Condominium Development
25mm Design Storm Event – Proposed Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

Element Count

Number of rain gages 1
Number of subcatchments ... 8
Number of nodes 13
Number of links 20
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-25mm	INTENSITY	5 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
EXT1	2.73	182.00	5.00	1.0000	London	OF1
P1	1.62	405.75	62.00	2.0000	London	ST3
P2	0.77	220.00	62.00	2.0000	London	CBMH20
P3	0.66	188.00	62.00	2.0000	London	ST10
P4	0.25	124.50	0.00	20.0000	London	DB_INFL
P5	1.24	311.00	62.00	2.0000	London	ST2
ROW1	0.34	169.00	79.00	2.0000	London	OF1
ROW2	0.06	24.00	100.00	0.2000	London	ST4

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
CBMH18	JUNCTION	283.61	2.53	0.0	
CBMH19	JUNCTION	283.84	2.31	0.0	
CBMH20	JUNCTION	284.04	2.14	0.0	
ST10	JUNCTION	284.16	2.11	0.0	
ST2	JUNCTION	282.60	1.86	0.0	
ST3	JUNCTION	282.69	2.15	0.0	
ST4	JUNCTION	282.83	2.53	0.0	
ST5	JUNCTION	283.13	2.53	0.0	
ST6	JUNCTION	283.36	2.60	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	281.75	0.00	0.0	
OF3	OUTFALL	283.00	0.95	0.0	
DB_INFL	STORAGE	281.75	2.35	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
15	CBMH20	CBMH19	CONDUIT	33.5	0.5075	0.0130
16	CBMH19	CBMH18	CONDUIT	40.7	0.4914	0.0130
19	ST5	ST4	CONDUIT	37.5	0.4000	0.0130
2	DB_INFL	OF3	CONDUIT	15.0	2.6676	0.0300
3	ST10	CBMH20	CONDUIT	18.3	0.4918	0.0130
4	CBMH18	ST6	CONDUIT	34.2	0.4971	0.0130
5	ST6	ST5	CONDUIT	30.7	0.4886	0.0130
6	ST4	ST3	CONDUIT	31.5	0.1905	0.0130
7	ST3	ST2	CONDUIT	36.7	0.1907	0.0130
8	ST2	DB_INFL	CONDUIT	15.4	0.3896	0.0130
10	CBMH18	ST6	WEIR			
11	ST6	ST5	WEIR			

DEL19-103: Watson Farm Condominium Development
25mm Design Storm Event – Proposed Conditions PCSWMM Output

12	ST4	ST3	WEIR
13	ST3	ST2	WEIR
14	ST2	DB_INFL	WEIR
17	CBMH20	CBMH19	WEIR
18	CBMH19	CBMH18	WEIR
20	ST5	ST4	WEIR
9	ST10	CBMH20	WEIR
1	DB_INFL	OF2	OUTLET

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
15	CIRCULAR	0.30	0.07	0.07	0.30	1	68.89
16	CIRCULAR	0.30	0.07	0.07	0.30	1	67.79
19	CIRCULAR	0.45	0.16	0.11	0.45	1	180.33
2	CIRCULAR	0.45	0.16	0.11	0.45	1	201.80
3	CIRCULAR	0.30	0.07	0.07	0.30	1	67.82
4	CIRCULAR	0.30	0.07	0.07	0.30	1	68.18
5	CIRCULAR	0.45	0.16	0.11	0.45	1	199.30
6	CIRCULAR	0.60	0.28	0.15	0.60	1	267.99
7	CIRCULAR	0.68	0.36	0.17	0.68	1	367.14
8	CIRCULAR	0.68	0.36	0.17	0.68	1	524.72

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units LPS
Process Models:
Rainfall/Runoff YES
RDII NO
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed YES
Water Quality NO
Infiltration Method CURVE_NUMBER
Flow Routing Method DYNWAVE
Starting Date 04/10/2015 00:00:00
Ending Date 04/13/2015 00:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:01:00
Dry Time Step 00:01:00
Routing Time Step 0.50 sec
Variable Time Step YES
Maximum Trials 8
Number of Threads 2
Head Tolerance 0.001500 m

	Volume	Depth
	hectare-m	mm
Runoff Quantity Continuity	0.192	25.000
Evaporation Loss	0.000	0.000
Infiltration Loss	0.108	14.020
Surface Runoff	0.074	9.637
Final Storage	0.010	1.355
Continuity Error (%)	-0.047	

	Volume	Volume
	hectare-m	10^6 ltr
Flow Routing Continuity	0.000	0.000
Dry Weather Inflow	0.074	0.739
Wet Weather Inflow	0.000	0.000
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000

DEL19-103: Watson Farm Condominium Development
 25mm Design Storm Event – Proposed Conditions PCSWMM Output

External Inflow	0.000	0.000
External Outflow	0.074	0.740
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	-0.171	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 0.50 sec
 Average Time Step : 0.50 sec
 Maximum Time Step : 0.50 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
EXT1	25.00	0.00	0.00	22.48	1.25	0.03	29.47	0.050
P1	25.00	0.00	0.00	9.00	14.60	0.24	142.63	0.584
P2	25.00	0.00	0.00	8.98	14.63	0.11	69.28	0.585
P3	25.00	0.00	0.00	8.98	14.63	0.10	59.20	0.585
P4	25.00	0.00	0.00	23.75	0.00	0.00	0.00	0.000
P5	25.00	0.00	0.00	9.00	14.60	0.18	109.33	0.584
ROW1	25.00	0.00	0.00	4.85	18.73	0.06	57.46	0.749
ROW2	25.00	0.00	0.00	0.00	24.26	0.01	11.15	0.971

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
CBMH18	JUNCTION	0.02	0.44	284.05	0 01:27	0.44
CBMH19	JUNCTION	0.02	0.65	284.49	0 01:26	0.65
CBMH20	JUNCTION	0.02	0.85	284.89	0 01:25	0.84
ST10	JUNCTION	0.01	0.77	284.93	0 01:25	0.76
ST2	JUNCTION	0.02	0.39	282.99	0 01:25	0.39
ST3	JUNCTION	0.02	0.39	283.08	0 01:25	0.39
ST4	JUNCTION	0.02	0.28	283.11	0 01:25	0.28
ST5	JUNCTION	0.01	0.25	283.38	0 01:27	0.25
ST6	JUNCTION	0.01	0.23	283.59	0 01:27	0.23
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	281.75	0 00:00	0.00
OF3	OUTFALL	0.00	0.00	283.00	0 00:00	0.00
DB_INFL	STORAGE	0.04	0.66	282.41	0 02:58	0.66

 Node Inflow Summary

DEL19-103: Watson Farm Condominium Development
 25mm Design Storm Event – Proposed Conditions PCSWMM Output

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
CBMH18	JUNCTION	0.00	102.10	0 01:26	0	0.208	-0.027
CBMH19	JUNCTION	0.00	109.63	0 01:25	0	0.208	0.191
CBMH20	JUNCTION	69.28	121.15	0 01:25	0.113	0.209	0.211
ST10	JUNCTION	59.20	59.20	0 01:25	0.0962	0.0962	0.018
ST2	JUNCTION	109.33	338.03	0 01:25	0.182	0.641	-0.099
ST3	JUNCTION	142.63	234.99	0 01:25	0.237	0.46	0.116
ST4	JUNCTION	11.15	107.09	0 01:27	0.0146	0.223	-0.156
ST5	JUNCTION	0.00	99.50	0 01:27	0	0.208	0.004
ST6	JUNCTION	0.00	99.44	0 01:27	0	0.208	0.004
OF1	OUTFALL	86.93	86.93	0 01:25	0.0974	0.0974	0.000
OF2	OUTFALL	0.00	22.91	0 01:46	0	0.643	0.000
OF3	OUTFALL	0.00	0.00	0 00:00	0	0	0.000 ltr
DB_INFL	STORAGE	0.00	337.92	0 01:25	0	0.642	-0.235

 Node Surge Summary

No nodes were surcharged.

 Node Flooding Summary

No nodes were flooded.

 Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
DB_INFL	0.026	1	0	0	0.398	17	0 02:58	22.91

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	6.59	5.70	86.93	0.097
OF2	11.20	22.15	22.91	0.643
OF3	0.00	0.00	0.00	0.000
System	5.93	27.85	108.97	0.740

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
15	CONDUIT	109.63	0 01:25	1.55	1.59	1.00
16	CONDUIT	102.10	0 01:26	1.44	1.51	1.00
19	CONDUIT	99.52	0 01:27	1.23	0.55	0.51
2	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
3	CONDUIT	52.29	0 01:25	0.81	0.77	1.00
4	CONDUIT	99.44	0 01:27	1.48	1.46	0.91
5	CONDUIT	99.50	0 01:27	1.28	0.50	0.49
6	CONDUIT	107.68	0 01:27	0.92	0.40	0.47

DEL19-103: Watson Farm Condominium Development
 25mm Design Storm Event – Proposed Conditions PCSWMM Output

7	CONDUIT	231.58	0	01:25	1.19	0.63	0.55			
8	CONDUIT	337.92	0	01:25	1.68	0.64	0.55			
10	WEIR	0.00	0	00:00			0.00			
11	WEIR	0.00	0	00:00			0.00			
12	WEIR	0.00	0	00:00			0.00			
13	WEIR	0.00	0	00:00			0.00			
14	WEIR	0.00	0	00:00			0.00			
17	WEIR	0.00	0	00:00			0.00			
18	WEIR	0.00	0	00:00			0.00			
20	WEIR	0.00	0	00:00			0.00			
9	WEIR	0.00	0	00:00			0.00			
1	DUMMY	22.91	0	01:46						

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class		Up		Down		Norm Ltd	Inlet Ctrl	
		Dry	Dry	Dry	Crit	Crit	Crit			
15	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.99	0.00	0.00
16	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.99	0.00	0.00
19	1.00	0.01	0.00	0.00	0.00	0.00	0.00	0.99	0.00	0.00
2	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	1.00	0.00	0.00	0.00	0.02	0.00	0.00	0.98	0.01	0.00
4	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00
5	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00
6	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.00
7	1.00	0.00	0.00	0.00	0.02	0.00	0.00	0.98	0.00	0.00
8	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00

 Conduit Surcharge Summary

Conduit	Hours Full		Hours Above Full		Hours Capacity Limited
	Both Ends	Upstream	Dnstream	Normal Flow	
15	0.16	0.18	0.16	0.20	0.16
16	0.13	0.18	0.13	0.21	0.13
3	0.15	0.15	0.17	0.01	0.01
4	0.01	0.16	0.01	0.21	0.01

Analysis begun on: Wed Jun 07 15:31:36 2023
 Analysis ended on: Wed Jun 07 15:31:48 2023
 Total elapsed time: 00:00:12

DEL19-103: Watson Farm Condominium Development
 2-year Design Storm Event – Proposed Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 8
 Number of nodes 13
 Number of links 20
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-2yr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
EXT1	2.73	182.00	5.00	1.0000	London	OF1
P1	1.62	405.75	62.00	2.0000	London	ST3
P2	0.77	220.00	62.00	2.0000	London	CBMH20
P3	0.66	188.00	62.00	2.0000	London	ST10
P4	0.25	124.50	0.00	20.0000	London	DB_INFL
P5	1.24	311.00	62.00	2.0000	London	ST2
ROW1	0.34	169.00	79.00	2.0000	London	OF1
ROW2	0.06	24.00	100.00	0.2000	London	ST4

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
CBMH18	JUNCTION	283.61	2.53	0.0	
CBMH19	JUNCTION	283.84	2.31	0.0	
CBMH20	JUNCTION	284.04	2.14	0.0	
ST10	JUNCTION	284.16	2.11	0.0	
ST2	JUNCTION	282.60	1.86	0.0	
ST3	JUNCTION	282.69	2.15	0.0	
ST4	JUNCTION	282.83	2.53	0.0	
ST5	JUNCTION	283.13	2.53	0.0	
ST6	JUNCTION	283.36	2.60	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	281.75	0.00	0.0	
OF3	OUTFALL	283.00	0.95	0.0	
DB_INFL	STORAGE	281.75	2.35	0.0	

 Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
15	CBMH20	CBMH19	CONDUIT	33.5	0.5075	0.0130
16	CBMH19	CBMH18	CONDUIT	40.7	0.4914	0.0130
19	ST5	ST4	CONDUIT	37.5	0.4000	0.0130
2	DB_INFL	OF3	CONDUIT	15.0	2.6676	0.0300
3	ST10	CBMH20	CONDUIT	18.3	0.4918	0.0130
4	CBMH18	ST6	CONDUIT	34.2	0.4971	0.0130
5	ST6	ST5	CONDUIT	30.7	0.4886	0.0130
6	ST4	ST3	CONDUIT	31.5	0.1905	0.0130
7	ST3	ST2	CONDUIT	36.7	0.1907	0.0130
8	ST2	DB_INFL	CONDUIT	15.4	0.3896	0.0130
10	CBMH18	ST6	WEIR			
11	ST6	ST5	WEIR			

DEL19-103: Watson Farm Condominium Development
 2-year Design Storm Event – Proposed Conditions PCSWMM Output

12	ST4	ST3	WEIR
13	ST3	ST2	WEIR
14	ST2	DB_INFL	WEIR
17	CBMH20	CBMH19	WEIR
18	CBMH19	CBMH18	WEIR
20	ST5	ST4	WEIR
9	ST10	CBMH20	WEIR
1	DB_INFL	OF2	OUTLET

 Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
15	CIRCULAR	0.30	0.07	0.07	0.30	1	68.89
16	CIRCULAR	0.30	0.07	0.07	0.30	1	67.79
19	CIRCULAR	0.45	0.16	0.11	0.45	1	180.33
2	CIRCULAR	0.45	0.16	0.11	0.45	1	201.80
3	CIRCULAR	0.30	0.07	0.07	0.30	1	67.82
4	CIRCULAR	0.30	0.07	0.07	0.30	1	68.18
5	CIRCULAR	0.45	0.16	0.11	0.45	1	199.30
6	CIRCULAR	0.60	0.28	0.15	0.60	1	267.99
7	CIRCULAR	0.68	0.36	0.17	0.68	1	367.14
8	CIRCULAR	0.68	0.36	0.17	0.68	1	524.72

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed YES
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Flow Routing Method DYNWAVE
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/13/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00
 Routing Time Step 0.50 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 2
 Head Tolerance 0.001500 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.328	42.751
Evaporation Loss	0.000	0.000
Infiltration Loss	0.168	21.942
Surface Runoff	0.149	19.477
Final Storage	0.010	1.351
Continuity Error (%)	-0.047	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.149	1.494
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000

DEL19-103: Watson Farm Condominium Development
 2-year Design Storm Event – Proposed Conditions PCSWMM Output

External Inflow	0.000	0.000
External Outflow	0.150	1.496
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	-0.127	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 0.50 sec
 Average Time Step : 0.50 sec
 Maximum Time Step : 0.50 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10 ⁶ ltr	Peak Runoff LPS	Runoff Coeff
EXT1	42.75	0.00	0.00	36.19	5.29	0.14	48.22	0.124
P1	42.75	0.00	0.00	13.50	27.87	0.45	279.46	0.652
P2	42.75	0.00	0.00	13.46	27.91	0.21	136.80	0.653
P3	42.75	0.00	0.00	13.46	27.91	0.18	116.90	0.653
P4	42.75	0.00	0.00	36.76	4.73	0.01	3.45	0.111
P5	42.75	0.00	0.00	13.50	27.87	0.35	214.20	0.652
ROW1	42.75	0.00	0.00	7.28	34.06	0.12	94.22	0.797
ROW2	42.75	0.00	0.00	0.00	42.02	0.03	19.34	0.983

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
CBMH18	JUNCTION	0.02	1.87	285.48	0 01:15	1.86
CBMH19	JUNCTION	0.02	2.18	286.02	0 01:15	2.18
CBMH20	JUNCTION	0.02	2.02	286.06	0 01:10	2.02
ST10	JUNCTION	0.02	1.97	286.13	0 01:10	1.97
ST2	JUNCTION	0.05	0.57	283.17	0 01:15	0.57
ST3	JUNCTION	0.04	0.61	283.30	0 01:15	0.61
ST4	JUNCTION	0.03	0.51	283.34	0 01:15	0.51
ST5	JUNCTION	0.02	0.43	283.56	0 01:15	0.43
ST6	JUNCTION	0.02	0.39	283.75	0 01:15	0.39
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	281.75	0 00:00	0.00
OF3	OUTFALL	0.00	0.00	283.00	0 00:00	0.00
DB_INFL	STORAGE	0.17	1.36	283.11	0 03:03	1.36

 Node Inflow Summary

DEL19-103: Watson Farm Condominium Development
 2-year Design Storm Event – Proposed Conditions PCSWMM Output

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10 ⁶ ltr	Total Inflow Volume 10 ⁶ ltr	Flow Balance Error Percent
CBMH18	JUNCTION	0.00	249.50	0 01:10	0	0.397	-0.001
CBMH19	JUNCTION	0.00	252.59	0 01:10	0	0.398	0.134
CBMH20	JUNCTION	136.80	253.61	0 01:10	0.215	0.399	0.180
ST10	JUNCTION	116.90	116.90	0 01:10	0.184	0.184	0.010
ST2	JUNCTION	214.20	664.57	0 01:15	0.347	1.22	-0.097
ST3	JUNCTION	279.46	475.67	0 01:15	0.452	0.875	0.058
ST4	JUNCTION	19.34	229.71	0 01:16	0.0252	0.424	0.164
ST5	JUNCTION	0.00	217.22	0 01:15	0	0.397	-0.272
ST6	JUNCTION	0.00	217.58	0 01:15	0	0.397	0.023
OF1	OUTFALL	142.43	142.43	0 01:10	0.26	0.26	0.000
OF2	OUTFALL	0.00	22.91	0 01:13	0	1.24	0.000
OF3	OUTFALL	0.00	0.00	0 00:00	0	0	0.000 ltr
DB_INFL	STORAGE	3.45	663.96	0 01:15	0.0118	1.23	-0.168

 Node Surge Summary

No nodes were surcharged.

 Node Flooding Summary

No nodes were flooded.

 Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
DB_INFL	0.112	5	0	0	0.997	43	0 03:03	22.91

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10 ⁶ ltr
OF1	7.50	13.35	142.43	0.260
OF2	21.16	22.54	22.91	1.237
OF3	0.00	0.00	0.00	0.000
System	9.55	35.89	164.84	1.496

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
15	CONDUIT	169.04	0 01:08	2.39	2.45	1.00
16	CONDUIT	184.88	0 01:10	2.62	2.73	1.00
19	CONDUIT	218.37	0 01:16	1.56	1.21	0.85
2	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
3	CONDUIT	83.51	0 01:08	1.18	1.23	1.00
4	CONDUIT	217.58	0 01:15	3.08	3.19	1.00
5	CONDUIT	217.22	0 01:15	1.62	1.09	0.80
6	CONDUIT	237.97	0 01:16	0.99	0.89	0.85

DEL19-103: Watson Farm Condominium Development
 2-year Design Storm Event – Proposed Conditions PCSWMM Output

7	CONDUIT	473.99	0	01:15	1.48	1.29	0.85			
8	CONDUIT	663.96	0	01:15	2.17	1.27	0.80			
10	WEIR	0.00	0	00:00			0.00			
11	WEIR	0.00	0	00:00			0.00			
12	WEIR	0.00	0	00:00			0.00			
13	WEIR	0.00	0	00:00			0.00			
14	WEIR	0.00	0	00:00			0.00			
17	WEIR	197.42	0	01:10			0.25			
18	WEIR	112.10	0	01:15			0.17			
20	WEIR	0.00	0	00:00			0.00			
9	WEIR	55.40	0	01:10			0.11			
1	DUMMY	22.91	0	01:13						

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class		Up		Down		Norm Ltd	Inlet Ctrl	
		Dry	Dry	Down Dry	Sub Crit	Sup Crit	Up Crit			
15	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.00
16	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.00
19	1.00	0.00	0.00	0.00	0.04	0.00	0.00	0.96	0.03	0.00
2	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	1.00	0.00	0.00	0.00	0.03	0.00	0.00	0.97	0.01	0.00
4	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00
5	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00
6	1.00	0.00	0.00	0.00	0.09	0.00	0.00	0.91	0.01	0.00
7	1.00	0.00	0.00	0.00	0.11	0.00	0.00	0.89	0.01	0.00
8	1.00	0.00	0.00	0.00	0.11	0.00	0.00	0.89	0.01	0.00

 Conduit Surcharge Summary

Conduit	Hours Full		Hours Above Full		Hours Capacity Limited
	Both Ends	Upstream	Normal Flow	Dnstream	
15	0.44	0.46	0.44	0.31	0.28
16	0.42	0.45	0.42	0.48	0.42
19	0.01	0.01	0.01	0.13	0.01
3	0.44	0.44	0.45	0.03	0.03
4	0.15	0.44	0.15	0.48	0.15
5	0.01	0.01	0.01	0.09	0.01
7	0.01	0.01	0.01	0.16	0.01
8	0.01	0.01	0.01	0.16	0.01

Analysis begun on: Wed Jun 07 15:38:22 2023
 Analysis ended on: Wed Jun 07 15:38:34 2023
 Total elapsed time: 00:00:12

DEL19-103: Watson Farm Condominium Development
5-year Design Storm Event – Proposed Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

Element Count

Number of rain gages 1
Number of subcatchments ... 8
Number of nodes 13
Number of links 20
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-Syr	INTENSITY	5 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
EXT1	2.73	182.00	5.00	1.0000	London	OF1
P1	1.62	405.75	62.00	2.0000	London	ST3
P2	0.77	220.00	62.00	2.0000	London	CBMH20
P3	0.66	188.00	62.00	2.0000	London	ST10
P4	0.25	124.50	0.00	20.0000	London	DB_INFL
P5	1.24	311.00	62.00	2.0000	London	ST2
ROW1	0.34	169.00	79.00	2.0000	London	OF1
ROW2	0.06	24.00	100.00	0.2000	London	ST4

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
CBMH18	JUNCTION	283.61	2.53	0.0	
CBMH19	JUNCTION	283.84	2.31	0.0	
CBMH20	JUNCTION	284.04	2.14	0.0	
ST10	JUNCTION	284.16	2.11	0.0	
ST2	JUNCTION	282.60	1.86	0.0	
ST3	JUNCTION	282.69	2.15	0.0	
ST4	JUNCTION	282.83	2.53	0.0	
ST5	JUNCTION	283.13	2.53	0.0	
ST6	JUNCTION	283.36	2.60	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	281.75	0.00	0.0	
OF3	OUTFALL	283.00	0.95	0.0	
DB_INFL	STORAGE	281.75	2.35	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
15	CBMH20	CBMH19	CONDUIT	33.5	0.5075	0.0130
16	CBMH19	CBMH18	CONDUIT	40.7	0.4914	0.0130
19	ST5	ST4	CONDUIT	37.5	0.4000	0.0130
2	DB_INFL	OF3	CONDUIT	15.0	2.6676	0.0300
3	ST10	CBMH20	CONDUIT	18.3	0.4918	0.0130
4	CBMH18	ST6	CONDUIT	34.2	0.4971	0.0130
5	ST6	ST5	CONDUIT	30.7	0.4886	0.0130
6	ST4	ST3	CONDUIT	31.5	0.1905	0.0130
7	ST3	ST2	CONDUIT	36.7	0.1907	0.0130
8	ST2	DB_INFL	CONDUIT	15.4	0.3896	0.0130
10	CBMH18	ST6	WEIR			
11	ST6	ST5	WEIR			

DEL19-103: Watson Farm Condominium Development
5-year Design Storm Event – Proposed Conditions PCSWMM Output

12	ST4	ST3	WEIR
13	ST3	ST2	WEIR
14	ST2	DB_INFL	WEIR
17	CBMH20	CBMH19	WEIR
18	CBMH19	CBMH18	WEIR
20	ST5	ST4	WEIR
9	ST10	CBMH20	WEIR
1	DB_INFL	OF2	OUTLET

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
15	CIRCULAR	0.30	0.07	0.07	0.30	1	68.89
16	CIRCULAR	0.30	0.07	0.07	0.30	1	67.79
19	CIRCULAR	0.45	0.16	0.11	0.45	1	180.33
2	CIRCULAR	0.45	0.16	0.11	0.45	1	201.80
3	CIRCULAR	0.30	0.07	0.07	0.30	1	67.82
4	CIRCULAR	0.30	0.07	0.07	0.30	1	68.18
5	CIRCULAR	0.45	0.16	0.11	0.45	1	199.30
6	CIRCULAR	0.60	0.28	0.15	0.60	1	267.99
7	CIRCULAR	0.68	0.36	0.17	0.68	1	367.14
8	CIRCULAR	0.68	0.36	0.17	0.68	1	524.72

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units LPS
Process Models:
Rainfall/Runoff YES
RDII NO
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed YES
Water Quality NO
Infiltration Method CURVE_NUMBER
Flow Routing Method DYNWAVE
Starting Date 04/10/2015 00:00:00
Ending Date 04/13/2015 00:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:01:00
Dry Time Step 00:01:00
Routing Time Step 0.50 sec
Variable Time Step YES
Maximum Trials 8
Number of Threads 2
Head Tolerance 0.001500 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.348	45.373
Evaporation Loss	0.000	0.000
Infiltration Loss	0.176	22.947
Surface Runoff	0.162	21.099
Final Storage	0.010	1.348
Continuity Error (%)	-0.050	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.162	1.619
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000

DEL19-103: Watson Farm Condominium Development
 5-year Design Storm Event – Proposed Conditions PCSWMM Output

External Inflow	0.000	0.000
External Outflow	0.162	1.620
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	-0.113	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 0.50 sec
 Average Time Step : 0.50 sec
 Maximum Time Step : 0.50 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
EXT1	45.37	0.00	0.00	37.92	6.19	0.17	52.18	0.137
P1	45.37	0.00	0.00	14.08	29.91	0.49	308.55	0.659
P2	45.37	0.00	0.00	14.05	29.95	0.23	151.05	0.660
P3	45.37	0.00	0.00	14.05	29.95	0.20	129.08	0.660
P4	45.37	0.00	0.00	38.29	5.83	0.01	4.63	0.129
P5	45.37	0.00	0.00	14.08	29.91	0.37	236.50	0.659
ROW1	45.37	0.00	0.00	7.60	36.37	0.12	101.98	0.802
ROW2	45.37	0.00	0.00	0.00	44.65	0.03	21.05	0.984

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
CBMH18	JUNCTION	0.02	2.28	285.89	0 01:15	2.28
CBMH19	JUNCTION	0.02	2.19	286.03	0 01:15	2.19
CBMH20	JUNCTION	0.02	2.02	286.06	0 01:10	2.02
ST10	JUNCTION	0.02	1.97	286.13	0 01:10	1.97
ST2	JUNCTION	0.06	0.61	283.21	0 01:15	0.61
ST3	JUNCTION	0.05	0.66	283.35	0 01:15	0.66
ST4	JUNCTION	0.03	0.56	283.39	0 01:15	0.56
ST5	JUNCTION	0.02	0.50	283.63	0 01:15	0.50
ST6	JUNCTION	0.02	0.47	283.83	0 01:15	0.46
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	281.75	0 00:00	0.00
OF3	OUTFALL	0.00	0.00	283.00	0 00:00	0.00
DB_INFL	STORAGE	0.19	1.44	283.19	0 03:04	1.44

 Node Inflow Summary

DEL19-103: Watson Farm Condominium Development
 5-year Design Storm Event – Proposed Conditions PCSWMM Output

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
CBMH18	JUNCTION	0.00	279.11	0 01:10	0	0.427	0.081
CBMH19	JUNCTION	0.00	279.91	0 01:10	0	0.427	0.124
CBMH20	JUNCTION	151.05	280.08	0 01:10	0.231	0.428	0.144
ST10	JUNCTION	129.08	129.08	0 01:10	0.197	0.197	0.010
ST2	JUNCTION	236.50	725.67	0 01:15	0.372	1.31	-0.092
ST3	JUNCTION	308.55	519.68	0 01:15	0.486	0.939	0.033
ST4	JUNCTION	21.05	251.16	0 01:15	0.0268	0.454	0.229
ST5	JUNCTION	0.00	237.23	0 01:15	0	0.426	-0.295
ST6	JUNCTION	0.00	237.82	0 01:15	0	0.426	0.021
OF1	OUTFALL	154.17	154.17	0 01:10	0.292	0.292	0.000
OF2	OUTFALL	0.00	22.91	0 01:12	0	1.33	0.000
OF3	OUTFALL	0.00	0.00	0 00:00	0	0	0.000 ltr
DB_INFL	STORAGE	4.63	725.23	0 01:15	0.0145	1.33	-0.166

 Node Surge Summary

No nodes were surcharged.

 Node Flooding Summary

No nodes were flooded.

 Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
DB_INFL	0.130	6	0	0	1.082	47	0 03:04	22.91

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	7.63	14.76	154.17	0.292
OF2	22.71	22.57	22.91	1.328
OF3	0.00	0.00	0.00	0.000
System	10.11	37.33	176.64	1.620

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
15	CONDUIT	172.04	0 01:08	2.43	2.50	1.00
16	CONDUIT	186.80	0 01:09	2.64	2.76	1.00
19	CONDUIT	238.30	0 01:15	1.57	1.32	0.95
2	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
3	CONDUIT	85.73	0 01:08	1.21	1.26	1.00
4	CONDUIT	237.82	0 01:15	3.36	3.49	1.00
5	CONDUIT	237.23	0 01:15	1.60	1.19	0.96
6	CONDUIT	259.17	0 01:16	1.00	0.97	0.93

DEL19-103: Watson Farm Condominium Development
 5-year Design Storm Event – Proposed Conditions PCSWMM Output

7	CONDUIT	518.43	0	01:15	1.53	1.41	0.91			
8	CONDUIT	725.10	0	01:15	2.26	1.38	0.90			
10	WEIR	0.00	0	00:00			0.00			
11	WEIR	0.00	0	00:00			0.00			
12	WEIR	0.00	0	00:00			0.00			
13	WEIR	0.00	0	00:00			0.00			
14	WEIR	0.00	0	00:00			0.00			
17	WEIR	239.60	0	01:10			0.28			
18	WEIR	185.07	0	01:15			0.24			
20	WEIR	0.00	0	00:00			0.00			
9	WEIR	70.38	0	01:10			0.12			
1	DUMMY	22.91	0	01:12						

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class		Up		Down		Norm Ltd	Inlet Ctrl	
		Dry	Dry	Dry	Crit	Sup Crit	Up Crit			
15	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.00
16	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.00
19	1.00	0.00	0.00	0.00	0.06	0.00	0.00	0.94	0.05	0.00
2	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	1.00	0.00	0.00	0.00	0.03	0.00	0.00	0.97	0.01	0.00
4	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00
5	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00
6	1.00	0.00	0.00	0.00	0.10	0.00	0.00	0.90	0.01	0.00
7	1.00	0.00	0.00	0.00	0.13	0.00	0.00	0.87	0.01	0.00
8	1.00	0.00	0.00	0.00	0.13	0.00	0.00	0.87	0.01	0.00

 Conduit Surcharge Summary

Conduit	Hours Full		Hours Above Full		Hours Capacity Limited
	Both Ends	Upstream	Dnstream	Normal Flow	
15	0.46	0.47	0.46	0.30	0.27
16	0.44	0.47	0.44	0.47	0.41
19	0.01	0.08	0.01	0.16	0.01
3	0.45	0.45	0.47	0.03	0.03
4	0.18	0.46	0.18	0.50	0.18
5	0.01	0.02	0.01	0.12	0.01
7	0.01	0.01	0.01	0.19	0.01
8	0.01	0.01	0.01	0.19	0.01

Analysis begun on: Wed Jun 07 15:39:04 2023
 Analysis ended on: Wed Jun 07 15:39:16 2023
 Total elapsed time: 00:00:12

DEL19-103: Watson Farm Condominium Development
 10-year Design Storm Event – Proposed Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 8
 Number of nodes 13
 Number of links 20
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-10yr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
EXT1	2.73	182.00	5.00	1.0000	London	OF1
P1	1.62	405.75	62.00	2.0000	London	ST3
P2	0.77	220.00	62.00	2.0000	London	CBMH20
P3	0.66	188.00	62.00	2.0000	London	ST10
P4	0.25	124.50	0.00	20.0000	London	DB_INFL
P5	1.24	311.00	62.00	2.0000	London	ST2
ROW1	0.34	169.00	79.00	2.0000	London	OF1
ROW2	0.06	24.00	100.00	0.2000	London	ST4

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
CBMH18	JUNCTION	283.61	2.53	0.0	
CBMH19	JUNCTION	283.84	2.31	0.0	
CBMH20	JUNCTION	284.04	2.14	0.0	
ST10	JUNCTION	284.16	2.11	0.0	
ST2	JUNCTION	282.60	1.86	0.0	
ST3	JUNCTION	282.69	2.15	0.0	
ST4	JUNCTION	282.83	2.53	0.0	
ST5	JUNCTION	283.13	2.53	0.0	
ST6	JUNCTION	283.36	2.60	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	281.75	0.00	0.0	
OF3	OUTFALL	283.00	0.95	0.0	
DB_INFL	STORAGE	281.75	2.35	0.0	

 Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
15	CBMH20	CBMH19	CONDUIT	33.5	0.5075	0.0130
16	CBMH19	CBMH18	CONDUIT	40.7	0.4914	0.0130
19	ST5	ST4	CONDUIT	37.5	0.4000	0.0130
2	DB_INFL	OF3	CONDUIT	15.0	2.6676	0.0300
3	ST10	CBMH20	CONDUIT	18.3	0.4918	0.0130
4	CBMH18	ST6	CONDUIT	34.2	0.4971	0.0130
5	ST6	ST5	CONDUIT	30.7	0.4886	0.0130
6	ST4	ST3	CONDUIT	31.5	0.1905	0.0130
7	ST3	ST2	CONDUIT	36.7	0.1907	0.0130
8	ST2	DB_INFL	CONDUIT	15.4	0.3896	0.0130
10	CBMH18	ST6	WEIR			
11	ST6	ST5	WEIR			

DEL19-103: Watson Farm Condominium Development
 10-year Design Storm Event – Proposed Conditions PCSWMM Output

12	ST4	ST3	WEIR
13	ST3	ST2	WEIR
14	ST2	DB_INFL	WEIR
17	CBMH20	CBMH19	WEIR
18	CBMH19	CBMH18	WEIR
20	ST5	ST4	WEIR
9	ST10	CBMH20	WEIR
1	DB_INFL	OF2	OUTLET

 Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
15	CIRCULAR	0.30	0.07	0.07	0.30	1	68.89
16	CIRCULAR	0.30	0.07	0.07	0.30	1	67.79
19	CIRCULAR	0.45	0.16	0.11	0.45	1	180.33
2	CIRCULAR	0.45	0.16	0.11	0.45	1	201.80
3	CIRCULAR	0.30	0.07	0.07	0.30	1	67.82
4	CIRCULAR	0.30	0.07	0.07	0.30	1	68.18
5	CIRCULAR	0.45	0.16	0.11	0.45	1	199.30
6	CIRCULAR	0.60	0.28	0.15	0.60	1	267.99
7	CIRCULAR	0.68	0.36	0.17	0.68	1	367.14
8	CIRCULAR	0.68	0.36	0.17	0.68	1	524.72

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed YES
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Flow Routing Method DYNWAVE
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/13/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00
 Routing Time Step 0.50 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 2
 Head Tolerance 0.001500 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.404	52.597
Evaporation Loss	0.000	0.000
Infiltration Loss	0.196	25.527
Surface Runoff	0.198	25.746
Final Storage	0.010	1.353
Continuity Error (%)	-0.056	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.198	1.975
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000

DEL19-103: Watson Farm Condominium Development
 10-year Design Storm Event – Proposed Conditions PCSWMM Output

External Inflow	0.000	0.000
External Outflow	0.198	1.978
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	-0.136	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 0.50 sec
 Average Time Step : 0.50 sec
 Maximum Time Step : 0.50 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
EXT1	52.60	0.00	0.00	42.31	9.02	0.25	63.30	0.171
P1	52.60	0.00	0.00	15.60	35.62	0.58	387.95	0.677
P2	52.60	0.00	0.00	15.56	35.67	0.27	189.89	0.678
P3	52.60	0.00	0.00	15.56	35.67	0.23	162.27	0.678
P4	52.60	0.00	0.00	42.26	9.09	0.02	8.98	0.173
P5	52.60	0.00	0.00	15.60	35.62	0.44	297.36	0.677
ROW1	52.60	0.00	0.00	8.43	42.78	0.14	123.15	0.813
ROW2	52.60	0.00	0.00	0.00	51.88	0.03	25.55	0.986

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
CBMH18	JUNCTION	0.02	2.39	286.00	0 01:10	2.39
CBMH19	JUNCTION	0.03	2.20	286.04	0 01:10	2.20
CBMH20	JUNCTION	0.03	2.03	286.07	0 01:10	2.03
ST10	JUNCTION	0.02	1.97	286.13	0 01:10	1.97
ST2	JUNCTION	0.09	0.80	283.40	0 03:05	0.80
ST3	JUNCTION	0.08	0.88	283.57	0 01:10	0.88
ST4	JUNCTION	0.06	0.84	283.67	0 01:11	0.84
ST5	JUNCTION	0.03	0.96	284.09	0 01:11	0.95
ST6	JUNCTION	0.02	1.08	284.44	0 01:11	1.07
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	281.75	0 00:00	0.00
OF3	OUTFALL	0.00	0.00	283.00	0 00:00	0.00
DB_INFL	STORAGE	0.26	1.65	283.40	0 03:05	1.65

 Node Inflow Summary

DEL19-103: Watson Farm Condominium Development
 10-year Design Storm Event – Proposed Conditions PCSWMM Output

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
CBMH18	JUNCTION	0.00	351.16	0 01:10	0	0.509	0.110
CBMH19	JUNCTION	0.00	351.82	0 01:10	0	0.509	0.061
CBMH20	JUNCTION	189.89	352.10	0 01:10	0.275	0.509	0.048
ST10	JUNCTION	162.27	162.27	0 01:10	0.235	0.235	0.006
ST2	JUNCTION	297.36	921.07	0 01:11	0.443	1.56	-0.080
ST3	JUNCTION	387.95	662.48	0 01:11	0.578	1.12	-0.005
ST4	JUNCTION	25.55	325.73	0 01:11	0.0311	0.54	0.137
ST5	JUNCTION	0.00	304.64	0 01:11	0	0.508	-0.207
ST6	JUNCTION	0.00	350.24	0 01:10	0	0.508	0.076
OF1	OUTFALL	186.45	186.45	0 01:10	0.391	0.391	0.000
OF2	OUTFALL	0.00	22.91	0 01:11	0	1.59	0.000
OF3	OUTFALL	0.00	0.00	0 00:00	0	0	0.000 ltr
DB_INFL	STORAGE	8.98	921.19	0 01:11	0.0226	1.58	-0.157

 Node Surge Summary

No nodes were surcharged.

 Node Flooding Summary

No nodes were flooded.

 Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
DB_INFL	0.187	8	0	0	1.322	57	0 03:05	22.91

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	7.95	18.95	186.45	0.391
OF2	27.06	22.63	22.91	1.587
OF3	0.00	0.00	0.00	0.000
System	11.67	41.58	209.13	1.978

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
15	CONDUIT	177.80	0 01:07	2.52	2.58	1.00
16	CONDUIT	190.93	0 01:08	2.70	2.82	1.00
19	CONDUIT	305.34	0 01:11	1.92	1.69	1.00
2	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
3	CONDUIT	90.32	0 01:07	1.28	1.33	1.00
4	CONDUIT	243.06	0 01:09	3.44	3.56	1.00
5	CONDUIT	304.64	0 01:11	1.92	1.53	1.00
6	CONDUIT	328.83	0 01:11	1.16	1.23	1.00

DEL19-103: Watson Farm Condominium Development
 10-year Design Storm Event – Proposed Conditions PCSWMM Output

7	CONDUIT	663.93	0	01:11	1.86	1.81	1.00			
8	CONDUIT	921.16	0	01:11	2.64	1.76	1.00			
10	WEIR	121.65	0	01:10						0.18
11	WEIR	0.00	0	00:00						0.00
12	WEIR	0.00	0	00:00						0.00
13	WEIR	0.00	0	00:00						0.00
14	WEIR	0.00	0	00:00						0.00
17	WEIR	319.36	0	01:10						0.36
18	WEIR	305.49	0	01:10						0.34
20	WEIR	0.00	0	00:00						0.00
9	WEIR	106.40	0	01:10						0.16
1	DUMMY	22.91	0	01:11						

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class		Up		Down		Norm Ltd	Inlet Ctrl	
		Dry	Dry	Dry	Crit	Crit	Crit			
15	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.00
16	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.00
19	1.00	0.00	0.00	0.00	0.10	0.00	0.00	0.89	0.03	0.00
2	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	1.00	0.00	0.00	0.00	0.03	0.00	0.00	0.97	0.02	0.00
4	1.00	0.00	0.00	0.00	0.00	0.00	0.00	1.00	0.00	0.00
5	1.00	0.00	0.00	0.00	0.05	0.00	0.00	0.94	0.05	0.00
6	1.00	0.00	0.00	0.00	0.15	0.00	0.00	0.85	0.01	0.00
7	1.00	0.00	0.00	0.00	0.17	0.00	0.00	0.83	0.01	0.00
8	1.00	0.00	0.00	0.00	0.17	0.00	0.00	0.83	0.01	0.00

 Conduit Surcharge Summary

Conduit	Hours Full		Hours Above Full		Hours Capacity Limited
	Both Ends	Upstream	Dnstream	Normal Flow	
15	0.51	0.52	0.51	0.33	0.27
16	0.49	0.52	0.49	0.43	0.36
19	0.12	0.15	0.12	0.21	0.12
3	0.50	0.50	0.51	0.02	0.02
4	0.23	0.51	0.23	0.57	0.23
5	0.13	0.14	0.13	0.19	0.13
6	0.12	0.12	0.95	0.12	0.11
7	1.08	1.11	2.71	0.24	0.09
8	3.00	3.11	4.06	0.24	0.01

Analysis begun on: Wed Jun 07 15:39:58 2023
 Analysis ended on: Wed Jun 07 15:40:10 2023
 Total elapsed time: 00:00:12

DEL19-103: Watson Farm Condominium Development
 25-year Design Storm Event – Proposed Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 8
 Number of nodes 13
 Number of links 20
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-25yr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
EXT1	2.73	182.00	5.00	1.0000	London	OF1
P1	1.62	405.75	62.00	2.0000	London	ST3
P2	0.77	220.00	62.00	2.0000	London	CBMH20
P3	0.66	188.00	62.00	2.0000	London	ST10
P4	0.25	124.50	0.00	20.0000	London	DB_INFL
P5	1.24	311.00	62.00	2.0000	London	ST2
ROW1	0.34	169.00	79.00	2.0000	London	OF1
ROW2	0.06	24.00	100.00	0.2000	London	ST4

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
CBMH18	JUNCTION	283.61	2.53	0.0	
CBMH19	JUNCTION	283.84	2.31	0.0	
CBMH20	JUNCTION	284.04	2.14	0.0	
ST10	JUNCTION	284.16	2.11	0.0	
ST2	JUNCTION	282.60	1.86	0.0	
ST3	JUNCTION	282.69	2.15	0.0	
ST4	JUNCTION	282.83	2.53	0.0	
ST5	JUNCTION	283.13	2.53	0.0	
ST6	JUNCTION	283.36	2.60	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	281.75	0.00	0.0	
OF3	OUTFALL	283.00	0.95	0.0	
DB_INFL	STORAGE	281.75	2.35	0.0	

 Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
15	CBMH20	CBMH19	CONDUIT	33.5	0.5075	0.0130
16	CBMH19	CBMH18	CONDUIT	40.7	0.4914	0.0130
19	ST5	ST4	CONDUIT	37.5	0.4000	0.0130
2	DB_INFL	OF3	CONDUIT	15.0	2.6676	0.0300
3	ST10	CBMH20	CONDUIT	18.3	0.4918	0.0130
4	CBMH18	ST6	CONDUIT	34.2	0.4971	0.0130
5	ST6	ST5	CONDUIT	30.7	0.4886	0.0130
6	ST4	ST3	CONDUIT	31.5	0.1905	0.0130
7	ST3	ST2	CONDUIT	36.7	0.1907	0.0130
8	ST2	DB_INFL	CONDUIT	15.4	0.3896	0.0130
10	CBMH18	ST6	WEIR			
11	ST6	ST5	WEIR			

DEL19-103: Watson Farm Condominium Development
 25-year Design Storm Event – Proposed Conditions PCSWMM Output

12	ST4	ST3	WEIR
13	ST3	ST2	WEIR
14	ST2	DB_INFL	WEIR
17	CBMH20	CBMH19	WEIR
18	CBMH19	CBMH18	WEIR
20	ST5	ST4	WEIR
9	ST10	CBMH20	WEIR
1	DB_INFL	OF2	OUTLET

 Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
15	CIRCULAR	0.30	0.07	0.07	0.30	1	68.89
16	CIRCULAR	0.30	0.07	0.07	0.30	1	67.79
19	CIRCULAR	0.45	0.16	0.11	0.45	1	180.33
2	CIRCULAR	0.45	0.16	0.11	0.45	1	201.80
3	CIRCULAR	0.30	0.07	0.07	0.30	1	67.82
4	CIRCULAR	0.30	0.07	0.07	0.30	1	68.18
5	CIRCULAR	0.45	0.16	0.11	0.45	1	199.30
6	CIRCULAR	0.60	0.28	0.15	0.60	1	267.99
7	CIRCULAR	0.68	0.36	0.17	0.68	1	367.14
8	CIRCULAR	0.68	0.36	0.17	0.68	1	524.72

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed YES
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Flow Routing Method DYNWAVE
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/13/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00
 Routing Time Step 0.50 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 2
 Head Tolerance 0.001500 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.463	60.381
Evaporation Loss	0.000	0.000
Infiltration Loss	0.216	28.152
Surface Runoff	0.237	30.926
Final Storage	0.010	1.346
Continuity Error (%)	-0.070	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.237	2.373
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000

DEL19-103: Watson Farm Condominium Development
 25-year Design Storm Event – Proposed Conditions PCSWMM Output

External Inflow	0.000	0.000
External Outflow	0.238	2.375
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	-0.116	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 0.50 sec
 Average Time Step : 0.50 sec
 Maximum Time Step : 0.50 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
EXT1	60.38	0.00	0.00	46.79	12.34	0.34	80.10	0.204
P1	60.38	0.00	0.00	17.15	41.88	0.68	497.73	0.694
P2	60.38	0.00	0.00	17.11	41.93	0.32	243.50	0.694
P3	60.38	0.00	0.00	17.11	41.93	0.28	208.08	0.694
P4	60.38	0.00	0.00	46.25	12.90	0.03	14.66	0.214
P5	60.38	0.00	0.00	17.15	41.88	0.52	381.50	0.694
ROW1	60.38	0.00	0.00	9.27	49.75	0.17	153.67	0.824
ROW2	60.38	0.00	0.00	0.00	59.68	0.04	31.73	0.988

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
CBMH18	JUNCTION	0.02	2.41	286.02	0 01:10	2.41
CBMH19	JUNCTION	0.03	2.22	286.06	0 01:10	2.22
CBMH20	JUNCTION	0.03	2.05	286.09	0 01:10	2.05
ST10	JUNCTION	0.03	1.98	286.14	0 01:10	1.98
ST2	JUNCTION	0.14	1.01	283.61	0 03:06	1.01
ST3	JUNCTION	0.12	1.20	283.89	0 01:10	1.19
ST4	JUNCTION	0.09	1.22	284.05	0 01:10	1.21
ST5	JUNCTION	0.05	1.63	284.76	0 01:10	1.63
ST6	JUNCTION	0.03	1.98	285.34	0 01:10	1.98
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	281.75	0 00:00	0.00
OF3	OUTFALL	0.00	0.00	283.00	0 00:00	0.00
DB_INFL	STORAGE	0.35	1.86	283.61	0 03:07	1.86

 Node Inflow Summary

DEL19-103: Watson Farm Condominium Development
 25-year Design Storm Event – Proposed Conditions PCSWMM Output

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
CBMH18	JUNCTION	0.00	451.00	0 01:10	0	0.598	0.156
CBMH19	JUNCTION	0.00	451.27	0 01:10	0	0.599	0.048
CBMH20	JUNCTION	243.50	451.51	0 01:10	0.323	0.599	0.039
ST10	JUNCTION	208.08	208.08	0 01:10	0.276	0.276	0.009
ST2	JUNCTION	381.50	1207.65	0 01:10	0.521	1.83	-0.048
ST3	JUNCTION	497.73	858.58	0 01:10	0.68	1.31	-0.009
ST4	JUNCTION	31.73	422.56	0 01:11	0.0358	0.634	0.022
ST5	JUNCTION	0.00	401.87	0 01:10	0	0.597	-0.133
ST6	JUNCTION	0.00	450.50	0 01:10	0	0.597	0.061
OF1	OUTFALL	233.77	233.77	0 01:10	0.505	0.505	0.000
OF2	OUTFALL	0.00	22.91	0 01:10	0	1.87	0.000
OF3	OUTFALL	0.00	0.00	0 00:00	0	0	0.000 ltr
DB_INFL	STORAGE	14.66	1210.44	0 01:10	0.0321	1.87	-0.151

 Node Surge Summary

No nodes were surcharged.

 Node Flooding Summary

No nodes were flooded.

 Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
DB_INFL	0.262	11	0	0	1.587	69	0 03:07	22.91

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	7.96	24.49	233.77	0.505
OF2	31.82	22.67	22.91	1.870
OF3	0.00	0.00	0.00	0.000
System	13.26	47.16	256.68	2.375

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
15	CONDUIT	182.77	0 01:06	2.59	2.65	1.00
16	CONDUIT	194.77	0 01:07	2.76	2.87	1.00
19	CONDUIT	397.40	0 01:11	2.50	2.20	1.00
2	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
3	CONDUIT	94.91	0 01:06	1.34	1.40	1.00
4	CONDUIT	242.25	0 01:08	3.43	3.55	1.00
5	CONDUIT	401.87	0 01:10	2.53	2.02	1.00
6	CONDUIT	433.62	0 01:11	1.53	1.62	1.00

DEL19-103: Watson Farm Condominium Development
 25-year Design Storm Event – Proposed Conditions PCSWMM Output

7	CONDUIT	864.20	0	01:10	2.41	2.35	1.00			
8	CONDUIT	1208.73	0	01:10	3.40	2.30	1.00			
10	WEIR	292.83	0	01:10						0.32
11	WEIR	0.00	0	00:00						0.00
12	WEIR	0.00	0	00:00						0.00
13	WEIR	0.00	0	00:00						0.00
14	WEIR	0.00	0	00:00						0.00
17	WEIR	420.70	0	01:10						0.46
18	WEIR	420.25	0	01:10						0.44
20	WEIR	0.00	0	00:00						0.00
9	WEIR	156.03	0	01:10						0.21
1	DUMMY	22.91	0	01:10						

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class		Up		Down		Norm Ltd	Inlet Ctrl	
		Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit			
15	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.00
16	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.00
19	1.00	0.00	0.00	0.00	0.15	0.00	0.00	0.84	0.04	0.00
2	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	1.00	0.00	0.00	0.00	0.03	0.00	0.00	0.97	0.02	0.00
4	1.00	0.00	0.00	0.00	0.05	0.00	0.00	0.95	0.05	0.00
5	1.00	0.00	0.00	0.00	0.11	0.00	0.00	0.89	0.04	0.00
6	1.00	0.00	0.00	0.00	0.19	0.00	0.00	0.81	0.01	0.00
7	1.00	0.00	0.00	0.00	0.22	0.00	0.00	0.78	0.01	0.00
8	1.00	0.00	0.00	0.00	0.22	0.00	0.00	0.78	0.01	0.00

 Conduit Surcharge Summary

Conduit	Hours Full		Hours Above Full		Hours Capacity Limited
	Both Ends	Upstream	Dnstream	Normal Flow	
15	0.54	0.57	0.54	0.34	0.28
16	0.52	0.57	0.52	0.44	0.35
19	0.96	1.00	4.33	0.24	0.16
3	0.53	0.53	0.55	0.02	0.02
4	0.27	0.54	0.27	0.61	0.27
5	0.17	0.18	0.17	0.22	0.17
6	4.33	4.33	5.42	0.16	0.15
7	5.47	5.50	6.61	0.27	0.14
8	6.78	6.94	7.68	0.27	0.01

Analysis begun on: Wed Jun 07 15:40:44 2023
 Analysis ended on: Wed Jun 07 15:40:55 2023
 Total elapsed time: 00:00:11

DEL19-103: Watson Farm Condominium Development
50-year Design Storm Event – Proposed Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

Element Count

Number of rain gages 1
Number of subcatchments ... 8
Number of nodes 13
Number of links 20
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-50yr	INTENSITY	5 min.

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
EXT1	2.73	182.00	5.00	1.0000	London	OF1
P1	1.62	405.75	62.00	2.0000	London	ST3
P2	0.77	220.00	62.00	2.0000	London	CBMH20
P3	0.66	188.00	62.00	2.0000	London	ST10
P4	0.25	124.50	0.00	20.0000	London	DB_INFL
P5	1.24	311.00	62.00	2.0000	London	ST2
ROW1	0.34	169.00	79.00	2.0000	London	OF1
ROW2	0.06	24.00	100.00	0.2000	London	ST4

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
CBMH18	JUNCTION	283.61	2.53	0.0	
CBMH19	JUNCTION	283.84	2.31	0.0	
CBMH20	JUNCTION	284.04	2.14	0.0	
ST10	JUNCTION	284.16	2.11	0.0	
ST2	JUNCTION	282.60	1.86	0.0	
ST3	JUNCTION	282.69	2.15	0.0	
ST4	JUNCTION	282.83	2.53	0.0	
ST5	JUNCTION	283.13	2.53	0.0	
ST6	JUNCTION	283.36	2.60	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	281.75	0.00	0.0	
OF3	OUTFALL	283.00	0.95	0.0	
DB_INFL	STORAGE	281.75	2.35	0.0	

Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
15	CBMH20	CBMH19	CONDUIT	33.5	0.5075	0.0130
16	CBMH19	CBMH18	CONDUIT	40.7	0.4914	0.0130
19	ST5	ST4	CONDUIT	37.5	0.4000	0.0130
2	DB_INFL	OF3	CONDUIT	15.0	2.6676	0.0300
3	ST10	CBMH20	CONDUIT	18.3	0.4918	0.0130
4	CBMH18	ST6	CONDUIT	34.2	0.4971	0.0130
5	ST6	ST5	CONDUIT	30.7	0.4886	0.0130
6	ST4	ST3	CONDUIT	31.5	0.1905	0.0130
7	ST3	ST2	CONDUIT	36.7	0.1907	0.0130
8	ST2	DB_INFL	CONDUIT	15.4	0.3896	0.0130
10	CBMH18	ST6	WEIR			
11	ST6	ST5	WEIR			

DEL19-103: Watson Farm Condominium Development
50-year Design Storm Event – Proposed Conditions PCSWMM Output

12	ST4	ST3	WEIR
13	ST3	ST2	WEIR
14	ST2	DB_INFL	WEIR
17	CBMH20	CBMH19	WEIR
18	CBMH19	CBMH18	WEIR
20	ST5	ST4	WEIR
9	ST10	CBMH20	WEIR
1	DB_INFL	OF2	OUTLET

Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
15	CIRCULAR	0.30	0.07	0.07	0.30	1	68.89
16	CIRCULAR	0.30	0.07	0.07	0.30	1	67.79
19	CIRCULAR	0.45	0.16	0.11	0.45	1	180.33
2	CIRCULAR	0.45	0.16	0.11	0.45	1	201.80
3	CIRCULAR	0.30	0.07	0.07	0.30	1	67.82
4	CIRCULAR	0.30	0.07	0.07	0.30	1	68.18
5	CIRCULAR	0.45	0.16	0.11	0.45	1	199.30
6	CIRCULAR	0.60	0.28	0.15	0.60	1	267.99
7	CIRCULAR	0.68	0.36	0.17	0.68	1	367.14
8	CIRCULAR	0.68	0.36	0.17	0.68	1	524.72

NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

Analysis Options

Flow Units LPS
Process Models:
Rainfall/Runoff YES
RDII NO
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed YES
Water Quality NO
Infiltration Method CURVE_NUMBER
Flow Routing Method DYNWAVE
Starting Date 04/10/2015 00:00:00
Ending Date 04/13/2015 00:00:00
Antecedent Dry Days 0.0
Report Time Step 00:01:00
Wet Time Step 00:01:00
Dry Time Step 00:01:00
Routing Time Step 0.50 sec
Variable Time Step YES
Maximum Trials 8
Number of Threads 2
Head Tolerance 0.001500 m

Volume Depth
Runoff Quantity Continuity hectare-m mm

Total Precipitation 0.507 66.122
Evaporation Loss 0.000 0.000
Infiltration Loss 0.229 29.912
Surface Runoff 0.268 34.905
Final Storage 0.010 1.357
Continuity Error (%) -0.078

Volume Volume
Flow Routing Continuity hectare-m 10^6 ltr

Dry Weather Inflow 0.000 0.000
Wet Weather Inflow 0.268 2.678
Groundwater Inflow 0.000 0.000
RDII Inflow 0.000 0.000

DEL19-103: Watson Farm Condominium Development
 50-year Design Storm Event – Proposed Conditions PCSWMM Output

External Inflow	0.000	0.000
External Outflow	0.268	2.680
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	-0.087	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 0.50 sec
 Average Time Step : 0.50 sec
 Maximum Time Step : 0.50 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
EXT1	66.12	0.00	0.00	49.77	15.08	0.41	93.08	0.228
P1	66.12	0.00	0.00	18.20	46.59	0.76	579.29	0.705
P2	66.12	0.00	0.00	18.16	46.64	0.36	283.26	0.705
P3	66.12	0.00	0.00	18.16	46.64	0.31	242.06	0.705
P4	66.12	0.00	0.00	48.96	15.91	0.04	21.84	0.241
P5	66.12	0.00	0.00	18.20	46.59	0.58	444.02	0.705
ROW1	66.12	0.00	0.00	9.83	54.94	0.19	175.87	0.831
ROW2	66.12	0.00	0.00	0.00	65.43	0.04	36.16	0.990

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
CBMH18	JUNCTION	0.03	2.42	286.03	0 01:10	2.42
CBMH19	JUNCTION	0.03	2.23	286.07	0 01:10	2.23
CBMH20	JUNCTION	0.03	2.06	286.10	0 01:10	2.06
ST10	JUNCTION	0.03	1.99	286.15	0 01:10	1.99
ST2	JUNCTION	0.18	1.16	283.76	0 03:06	1.16
ST3	JUNCTION	0.16	1.49	284.18	0 01:10	1.47
ST4	JUNCTION	0.13	1.57	284.40	0 01:10	1.55
ST5	JUNCTION	0.07	2.25	285.38	0 01:10	2.23
ST6	JUNCTION	0.04	2.47	285.83	0 01:10	2.46
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	281.75	0 00:00	0.00
OF3	OUTFALL	0.00	0.00	283.00	0 00:00	0.00
DB_INFL	STORAGE	0.42	2.01	283.76	0 03:08	2.01

 Node Inflow Summary

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Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
CBMH18	JUNCTION	0.00	524.52	0 01:10	0	0.665	0.125
CBMH19	JUNCTION	0.00	524.90	0 01:10	0	0.666	0.109
CBMH20	JUNCTION	283.26	525.24	0 01:10	0.359	0.666	0.040
ST10	JUNCTION	242.06	242.06	0 01:10	0.307	0.307	0.007
ST2	JUNCTION	444.02	1399.00	0 01:10	0.58	2.04	-0.051
ST3	JUNCTION	579.29	1005.17	0 01:10	0.756	1.46	-0.014
ST4	JUNCTION	36.16	491.89	0 01:10	0.0393	0.703	-0.050
ST5	JUNCTION	0.00	522.25	0 01:10	0	0.663	-0.008
ST6	JUNCTION	0.00	523.99	0 01:10	0	0.664	0.108
OF1	OUTFALL	268.94	268.94	0 01:10	0.597	0.597	0.000
OF2	OUTFALL	0.00	22.91	0 01:09	0	2.08	0.000
OF3	OUTFALL	0.00	0.00	0 00:00	0	0	0.000 ltr
DB_INFL	STORAGE	21.84	1405.87	0 01:10	0.0396	2.08	-0.150

 Node Surge Summary

No nodes were surcharged.

 Node Flooding Summary

No nodes were flooded.

 Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
DB_INFL	0.326	14	0	0	1.787	78	0 03:08	22.91

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	8.02	28.75	268.94	0.597
OF2	35.39	22.70	22.91	2.083
OF3	0.00	0.00	0.00	0.000
System	14.47	51.45	291.85	2.680

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
15	CONDUIT	184.55	0 01:06	2.61	2.68	1.00
16	CONDUIT	196.37	0 01:07	2.78	2.90	1.00
19	CONDUIT	461.93	0 01:10	2.90	2.56	1.00
2	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
3	CONDUIT	96.93	0 01:06	1.37	1.43	1.00
4	CONDUIT	241.63	0 01:08	3.42	3.54	1.00
5	CONDUIT	456.82	0 01:10	2.87	2.29	1.00
6	CONDUIT	508.20	0 01:11	1.80	1.90	1.00

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 50-year Design Storm Event – Proposed Conditions PCSWMM Output

7	CONDUIT	1008.40	0	01:10	2.82	2.75	1.00			
8	CONDUIT	1398.71	0	01:10	3.91	2.67	1.00			
10	WEIR	438.48	0	01:10						0.42
11	WEIR	114.65	0	01:10						0.17
12	WEIR	0.00	0	00:00						0.00
13	WEIR	0.00	0	00:00						0.00
14	WEIR	0.00	0	00:00						0.00
17	WEIR	494.70	0	01:10						0.54
18	WEIR	495.28	0	01:10						0.52
20	WEIR	0.00	0	00:00						0.00
9	WEIR	192.92	0	01:10						0.24
1	DUMMY	22.91	0	01:09						

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class		Up		Down		Norm Ltd	Inlet Ctrl	
		Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit			
15	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.00
16	1.00	0.00	0.00	0.00	0.04	0.00	0.00	0.96	0.03	0.00
19	1.00	0.00	0.00	0.00	0.19	0.00	0.00	0.81	0.04	0.00
2	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	1.00	0.00	0.00	0.00	0.03	0.00	0.00	0.97	0.02	0.00
4	1.00	0.00	0.00	0.00	0.09	0.00	0.00	0.91	0.05	0.00
5	1.00	0.00	0.00	0.00	0.15	0.00	0.00	0.85	0.04	0.00
6	1.00	0.00	0.00	0.00	0.23	0.00	0.00	0.77	0.01	0.00
7	1.00	0.00	0.00	0.00	0.26	0.00	0.00	0.74	0.01	0.00
8	1.00	0.00	0.00	0.00	0.25	0.00	0.00	0.75	0.01	0.00

 Conduit Surcharge Summary

Conduit	Hours Full		Hours Above Full		Hours Capacity Limited
	Both Ends	Upstream	Dnstream	Normal Flow	
15	0.59	0.61	0.59	0.35	0.30
16	0.55	0.61	0.55	0.44	0.35
19	4.46	4.50	7.22	0.25	0.19
3	0.57	0.57	0.59	0.02	0.02
4	0.29	0.58	0.69	0.65	0.29
5	0.19	0.21	2.74	0.24	0.19
6	7.21	7.22	8.20	0.19	0.18
7	8.26	8.29	9.31	0.28	0.17
8	9.50	9.63	10.37	0.28	0.05

Analysis begun on: Wed Jun 07 15:41:23 2023
 Analysis ended on: Wed Jun 07 15:41:34 2023
 Total elapsed time: 00:00:11

DEL19-103: Watson Farm Condominium Development
 100-year Design Storm Event – Proposed Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 8
 Number of nodes 13
 Number of links 20
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-100yr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
EXT1	2.73	182.00	5.00	1.0000	London	OF1
P1	1.62	405.75	62.00	2.0000	London	ST3
P2	0.77	220.00	62.00	2.0000	London	CBMH20
P3	0.66	188.00	62.00	2.0000	London	ST10
P4	0.25	124.50	0.00	20.0000	London	DB_INFL
P5	1.24	311.00	62.00	2.0000	London	ST2
ROW1	0.34	169.00	79.00	2.0000	London	OF1
ROW2	0.06	24.00	100.00	0.2000	London	ST4

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
CBMH18	JUNCTION	283.61	2.53	0.0	
CBMH19	JUNCTION	283.84	2.31	0.0	
CBMH20	JUNCTION	284.04	2.14	0.0	
ST10	JUNCTION	284.16	2.11	0.0	
ST2	JUNCTION	282.60	1.86	0.0	
ST3	JUNCTION	282.69	2.15	0.0	
ST4	JUNCTION	282.83	2.53	0.0	
ST5	JUNCTION	283.13	2.53	0.0	
ST6	JUNCTION	283.36	2.60	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	281.75	0.00	0.0	
OF3	OUTFALL	283.00	0.95	0.0	
DB_INFL	STORAGE	281.75	2.35	0.0	

 Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
15	CBMH20	CBMH19	CONDUIT	33.5	0.5075	0.0130
16	CBMH19	CBMH18	CONDUIT	40.7	0.4914	0.0130
19	ST5	ST4	CONDUIT	37.5	0.4000	0.0130
2	DB_INFL	OF3	CONDUIT	15.0	2.6676	0.0300
3	ST10	CBMH20	CONDUIT	18.3	0.4918	0.0130
4	CBMH18	ST6	CONDUIT	34.2	0.4971	0.0130
5	ST6	ST5	CONDUIT	30.7	0.4886	0.0130
6	ST4	ST3	CONDUIT	31.5	0.1905	0.0130
7	ST3	ST2	CONDUIT	36.7	0.1907	0.0130
8	ST2	DB_INFL	CONDUIT	15.4	0.3896	0.0130
10	CBMH18	ST6	WEIR			
11	ST6	ST5	WEIR			

DEL19-103: Watson Farm Condominium Development
 100-year Design Storm Event – Proposed Conditions PCSWMM Output

12	ST4	ST3	WEIR
13	ST3	ST2	WEIR
14	ST2	DB_INFL	WEIR
17	CBMH20	CBMH19	WEIR
18	CBMH19	CBMH18	WEIR
20	ST5	ST4	WEIR
9	ST10	CBMH20	WEIR
1	DB_INFL	OF2	OUTLET

 Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
15	CIRCULAR	0.30	0.07	0.07	0.30	1	68.89
16	CIRCULAR	0.30	0.07	0.07	0.30	1	67.79
19	CIRCULAR	0.45	0.16	0.11	0.45	1	180.33
2	CIRCULAR	0.45	0.16	0.11	0.45	1	201.80
3	CIRCULAR	0.30	0.07	0.07	0.30	1	67.82
4	CIRCULAR	0.30	0.07	0.07	0.30	1	68.18
5	CIRCULAR	0.45	0.16	0.11	0.45	1	199.30
6	CIRCULAR	0.60	0.28	0.15	0.60	1	267.99
7	CIRCULAR	0.68	0.36	0.17	0.68	1	367.14
8	CIRCULAR	0.68	0.36	0.17	0.68	1	524.72

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed YES
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Flow Routing Method DYNWAVE
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/13/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00
 Routing Time Step 0.50 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 2
 Head Tolerance 0.001500 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.551	71.801
Evaporation Loss	0.000	0.000
Infiltration Loss	0.242	31.585
Surface Runoff	0.299	38.925
Final Storage	0.010	1.354
Continuity Error (%)	-0.088	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.299	2.986
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000

DEL19-103: Watson Farm Condominium Development
 100-year Design Storm Event – Proposed Conditions PCSWMM Output

External Inflow	0.000	0.000
External Outflow	0.299	2.987
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	-0.028	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 0.50 sec
 Average Time Step : 0.50 sec
 Maximum Time Step : 0.50 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
EXT1	71.80	0.00	0.00	52.60	17.93	0.49	107.74	0.250
P1	71.80	0.00	0.00	19.20	51.29	0.83	667.24	0.714
P2	71.80	0.00	0.00	19.15	51.34	0.40	326.09	0.715
P3	71.80	0.00	0.00	19.15	51.34	0.34	278.66	0.715
P4	71.80	0.00	0.00	51.55	19.03	0.05	28.56	0.265
P5	71.80	0.00	0.00	19.20	51.29	0.64	511.43	0.714
ROW1	71.80	0.00	0.00	10.37	60.11	0.20	199.63	0.837
ROW2	71.80	0.00	0.00	0.00	71.13	0.04	40.83	0.991

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
CBMH18	JUNCTION	0.04	2.43	286.04	0 01:10	2.43
CBMH19	JUNCTION	0.03	2.24	286.08	0 01:10	2.24
CBMH20	JUNCTION	0.03	2.07	286.11	0 01:10	2.07
ST10	JUNCTION	0.03	1.99	286.15	0 01:10	1.99
ST2	JUNCTION	0.23	1.29	283.89	0 03:09	1.29
ST3	JUNCTION	0.20	1.96	284.65	0 01:10	1.87
ST4	JUNCTION	0.17	2.13	284.96	0 01:10	2.02
ST5	JUNCTION	0.10	2.41	285.54	0 01:10	2.41
ST6	JUNCTION	0.06	2.49	285.85	0 01:10	2.49
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	281.75	0 00:00	0.00
OF3	OUTFALL	0.00	0.00	283.00	0 00:00	0.00
DB_INFL	STORAGE	0.50	2.14	283.89	0 03:08	2.14

 Node Inflow Summary

DEL19-103: Watson Farm Condominium Development
 100-year Design Storm Event – Proposed Conditions PCSWMM Output

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
CBMH18	JUNCTION	0.00	604.24	0 01:10	0	0.732	0.312
CBMH19	JUNCTION	0.00	604.36	0 01:10	0	0.733	0.139
CBMH20	JUNCTION	326.09	604.66	0 01:10	0.395	0.733	0.037
ST10	JUNCTION	278.66	278.66	0 01:10	0.338	0.338	0.007
ST2	JUNCTION	511.43	1696.98	0 01:10	0.638	2.24	-0.025
ST3	JUNCTION	667.24	1240.63	0 01:10	0.832	1.6	-0.020
ST4	JUNCTION	40.83	642.35	0 01:10	0.0427	0.772	-0.085
ST5	JUNCTION	0.00	603.99	0 01:10	0	0.729	-0.027
ST6	JUNCTION	0.00	604.23	0 01:10	0	0.73	0.082
OF1	OUTFALL	307.37	307.37	0 01:10	0.693	0.693	0.000
OF2	OUTFALL	0.00	22.91	0 01:08	0	2.29	0.000
OF3	OUTFALL	0.00	0.00	0 00:00	0	0	0.000
DB_INFL	STORAGE	28.56	1716.13	0 01:10	0.0474	2.29	-0.138

 Node Surge Summary

No nodes were surcharged.

 Node Flooding Summary

No nodes were flooded.

 Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
DB_INFL	0.398	17	0	0	1.987	86	0 03:08	22.91

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	8.02	33.33	307.37	0.693
OF2	38.95	22.73	22.91	2.294
OF3	0.00	0.00	0.00	0.000
System	15.66	56.06	330.28	2.987

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
15	CONDUIT	185.71	0 01:06	2.63	2.70	1.00
16	CONDUIT	197.33	0 01:06	2.79	2.91	1.00
19	CONDUIT	472.04	0 01:12	2.97	2.62	1.00
2	CONDUIT	0.00	0 00:00	0.00	0.00	0.00
3	CONDUIT	99.16	0 01:06	1.40	1.46	1.00
4	CONDUIT	241.03	0 01:07	3.41	3.54	1.00
5	CONDUIT	465.21	0 01:09	2.93	2.33	1.00
6	CONDUIT	609.34	0 01:10	2.16	2.27	1.00

DEL19-103: Watson Farm Condominium Development
 100-year Design Storm Event – Proposed Conditions PCSWMM Output

7	CONDUIT	1229.11	0	01:10	3.43	3.35	1.00			
8	CONDUIT	1702.80	0	01:10	4.76	3.25	1.00			
10	WEIR	531.63	0	01:10						0.48
11	WEIR	313.54	0	01:10						0.34
12	WEIR	0.00	0	00:00						0.00
13	WEIR	0.00	0	00:00						0.00
14	WEIR	0.00	0	00:00						0.00
17	WEIR	574.16	0	01:10						0.61
18	WEIR	575.26	0	01:10						0.59
20	WEIR	210.83	0	01:10						0.26
9	WEIR	232.61	0	01:10						0.28
1	DUMMY	22.91	0	01:08						

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class		Up		Down		Norm Ltd	Inlet Ctrl	
		Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit			
15	1.00	0.00	0.00	0.00	0.01	0.00	0.00	0.99	0.00	0.00
16	1.00	0.00	0.00	0.00	0.09	0.00	0.00	0.91	0.07	0.00
19	1.00	0.00	0.00	0.00	0.23	0.00	0.00	0.77	0.06	0.00
2	1.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	1.00	0.00	0.00	0.00	0.04	0.00	0.00	0.96	0.02	0.00
4	1.00	0.00	0.00	0.00	0.13	0.00	0.00	0.87	0.05	0.00
5	1.00	0.00	0.00	0.00	0.18	0.00	0.00	0.81	0.05	0.00
6	1.00	0.00	0.00	0.00	0.26	0.00	0.00	0.74	0.01	0.00
7	1.00	0.00	0.00	0.00	0.29	0.00	0.00	0.71	0.01	0.00
8	1.00	0.00	0.00	0.00	0.29	0.00	0.00	0.71	0.02	0.00

 Conduit Surcharge Summary

Conduit	Hours Full		Hours Above Full		Hours Capacity Limited
	Both Ends	Upstream	Dnstream	Normal Flow	
15	0.62	0.64	0.62	0.37	0.31
16	0.58	0.64	0.58	0.45	0.36
19	7.52	7.52	9.93	0.27	0.28
3	0.60	0.60	0.63	0.02	0.01
4	0.30	0.61	4.25	0.67	0.30
5	2.59	2.59	5.92	0.25	0.25
6	9.93	9.93	10.85	0.19	0.19
7	10.92	10.92	11.96	0.30	0.28
8	12.15	12.25	13.01	0.29	0.17

Analysis begun on: Wed Jun 07 15:42:11 2023
 Analysis ended on: Wed Jun 07 15:42:22 2023
 Total elapsed time: 00:00:11

DEL19-103: Watson Farm Condominium Development
 250-year Design Storm Event – Proposed Conditions PCSWMM Output

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.012)

DEL19-103 - Watson Farm Lands

 Element Count

 Number of rain gages 1
 Number of subcatchments ... 8
 Number of nodes 13
 Number of links 20
 Number of pollutants 0
 Number of land uses 0

 Raingage Summary

Name	Data Source	Data Type	Recording Interval
London	London-250yr	INTENSITY	5 min.

 Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage	Outlet
EXT1	2.73	182.00	5.00	1.0000	London	OF1
P1	1.62	405.75	62.00	2.0000	London	ST3
P2	0.77	220.00	62.00	2.0000	London	CBMH20
P3	0.66	188.00	62.00	2.0000	London	ST10
P4	0.25	124.50	0.00	20.0000	London	DB_INFL
P5	1.24	311.00	62.00	2.0000	London	ST2
ROW1	0.34	169.00	79.00	2.0000	London	OF1
ROW2	0.06	24.00	100.00	0.2000	London	ST4

 Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
CBMH18	JUNCTION	283.61	2.53	0.0	
CBMH19	JUNCTION	283.84	2.31	0.0	
CBMH20	JUNCTION	284.04	2.14	0.0	
ST10	JUNCTION	284.16	2.11	0.0	
ST2	JUNCTION	282.60	1.86	0.0	
ST3	JUNCTION	282.69	2.15	0.0	
ST4	JUNCTION	282.83	2.53	0.0	
ST5	JUNCTION	283.13	2.53	0.0	
ST6	JUNCTION	283.36	2.60	0.0	
OF1	OUTFALL	0.00	0.00	0.0	
OF2	OUTFALL	281.75	0.00	0.0	
OF3	OUTFALL	283.00	0.95	0.0	
DB_INFL	STORAGE	281.75	2.35	0.0	

 Link Summary

Name	From Node	To Node	Type	Length	%Slope	Roughness
15	CBMH20	CBMH19	CONDUIT	33.5	0.5075	0.0130
16	CBMH19	CBMH18	CONDUIT	40.7	0.4914	0.0130
19	ST5	ST4	CONDUIT	37.5	0.4000	0.0130
2	DB_INFL	OF3	CONDUIT	15.0	2.6676	0.0300
3	ST10	CBMH20	CONDUIT	18.3	0.4918	0.0130
4	CBMH18	ST6	CONDUIT	34.2	0.4971	0.0130
5	ST6	ST5	CONDUIT	30.7	0.4886	0.0130
6	ST4	ST3	CONDUIT	31.5	0.1905	0.0130
7	ST3	ST2	CONDUIT	36.7	0.1907	0.0130
8	ST2	DB_INFL	CONDUIT	15.4	0.3896	0.0130
10	CBMH18	ST6	WEIR			
11	ST6	ST5	WEIR			

DEL19-103: Watson Farm Condominium Development
 250-year Design Storm Event – Proposed Conditions PCSWMM Output

12	ST4	ST3	WEIR
13	ST3	ST2	WEIR
14	ST2	DB_INFL	WEIR
17	CBMH20	CBMH19	WEIR
18	CBMH19	CBMH18	WEIR
20	ST5	ST4	WEIR
9	ST10	CBMH20	WEIR
1	DB_INFL	OF2	OUTLET

 Cross Section Summary

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
15	CIRCULAR	0.30	0.07	0.07	0.30	1	68.89
16	CIRCULAR	0.30	0.07	0.07	0.30	1	67.79
19	CIRCULAR	0.45	0.16	0.11	0.45	1	180.33
2	CIRCULAR	0.45	0.16	0.11	0.45	1	201.80
3	CIRCULAR	0.30	0.07	0.07	0.30	1	67.82
4	CIRCULAR	0.30	0.07	0.07	0.30	1	68.18
5	CIRCULAR	0.45	0.16	0.11	0.45	1	199.30
6	CIRCULAR	0.60	0.28	0.15	0.60	1	267.99
7	CIRCULAR	0.68	0.36	0.17	0.68	1	367.14
8	CIRCULAR	0.68	0.36	0.17	0.68	1	524.72

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units LPS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed YES
 Water Quality NO
 Infiltration Method CURVE_NUMBER
 Flow Routing Method DYNWAVE
 Starting Date 04/10/2015 00:00:00
 Ending Date 04/13/2015 00:00:00
 Antecedent Dry Days 0.0
 Report Time Step 00:01:00
 Wet Time Step 00:01:00
 Dry Time Step 00:01:00
 Routing Time Step 0.50 sec
 Variable Time Step YES
 Maximum Trials 8
 Number of Threads 2
 Head Tolerance 0.001500 m

	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
Total Precipitation	0.664	86.611
Evaporation Loss	0.000	0.000
Infiltration Loss	0.269	35.038
Surface Runoff	0.386	50.270
Final Storage	0.010	1.350
Continuity Error (%)	-0.054	

	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.386	3.857
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000

DEL19-103: Watson Farm Condominium Development
 250-year Design Storm Event – Proposed Conditions PCSWMM Output

External Inflow	0.000	0.000
External Outflow	0.386	3.856
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.013	

 Time-Step Critical Elements

 None

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

 Minimum Time Step : 0.50 sec
 Average Time Step : 0.50 sec
 Maximum Time Step : 0.50 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 2.00
 Percent Not Converging : 0.00

 Subcatchment Runoff Summary

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff LPS	Runoff Coeff
EXT1	86.61	0.00	0.00	58.17	27.18	0.74	114.95	0.314
P1	86.61	0.00	0.00	21.38	63.88	1.04	702.00	0.738
P2	86.61	0.00	0.00	21.34	63.93	0.49	342.12	0.738
P3	86.61	0.00	0.00	21.34	63.93	0.42	292.36	0.738
P4	86.61	0.00	0.00	57.44	27.95	0.07	46.50	0.323
P5	86.61	0.00	0.00	21.38	63.88	0.79	538.07	0.738
ROW1	86.61	0.00	0.00	11.61	73.64	0.25	201.12	0.850
ROW2	86.61	0.00	0.00	0.00	85.92	0.05	40.88	0.992

 Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
CBMH18	JUNCTION	0.06	2.43	286.04	0 01:10	2.43
CBMH19	JUNCTION	0.04	2.24	286.08	0 01:10	2.24
CBMH20	JUNCTION	0.03	2.07	286.11	0 01:10	2.07
ST10	JUNCTION	0.03	1.99	286.15	0 01:10	1.99
ST2	JUNCTION	0.27	1.49	284.09	0 02:18	1.49
ST3	JUNCTION	0.25	2.01	284.70	0 01:10	2.01
ST4	JUNCTION	0.21	2.26	285.09	0 01:10	2.26
ST5	JUNCTION	0.13	2.42	285.55	0 01:10	2.42
ST6	JUNCTION	0.09	2.49	285.85	0 01:10	2.49
OF1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
OF2	OUTFALL	0.00	0.00	281.75	0 00:00	0.00
OF3	OUTFALL	0.00	0.00	283.00	0 00:00	0.00
DB_INFL	STORAGE	0.56	2.34	284.09	0 02:20	2.34

 Node Inflow Summary

DEL19-103: Watson Farm Condominium Development
 250-year Design Storm Event – Proposed Conditions PCSWMM Output

Node	Type	Maximum Lateral Inflow LPS	Maximum Total Inflow LPS	Time of Max Occurrence days hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
CBMH18	JUNCTION	0.00	634.09	0 01:10	0	0.912	0.426
CBMH19	JUNCTION	0.00	634.19	0 01:10	0	0.912	0.032
CBMH20	JUNCTION	342.12	634.39	0 01:10	0.492	0.913	0.116
ST10	JUNCTION	292.36	292.36	0 01:10	0.421	0.421	0.005
ST2	JUNCTION	538.07	1899.66	0 01:10	0.795	2.79	-0.005
ST3	JUNCTION	702.00	1368.08	0 01:10	1.04	2	-0.014
ST4	JUNCTION	40.88	674.02	0 01:10	0.0516	0.959	-0.106
ST5	JUNCTION	0.00	634.01	0 01:10	0	0.907	-0.057
ST6	JUNCTION	0.00	634.09	0 01:10	0	0.908	0.068
OF1	OUTFALL	314.56	314.56	0 01:10	0.991	0.991	0.000
OF2	OUTFALL	0.00	22.91	0 01:07	0	2.45	0.000
OF3	OUTFALL	0.00	75.96	0 02:20	0	0.414	0.000
DB_INFL	STORAGE	46.50	1926.21	0 01:10	0.0696	2.86	-0.115

 Node Surge Summary

No nodes were surcharged.

 Node Flooding Summary

No nodes were flooded.

 Storage Volume Summary

Storage Unit	Average Volume 1000 m3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 m3	Max Pcnt Full	Time of Max Occurrence days hr:min	Maximum Outflow LPS
DB_INFL	0.464	20	0	0	2.287	99	0 02:20	98.87

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow LPS	Max Flow LPS	Total Volume 10^6 ltr
OF1	9.13	41.87	314.56	0.991
OF2	41.58	22.74	22.91	2.451
OF3	4.77	33.47	75.96	0.414
System	18.49	98.09	337.47	3.856

 Link Flow Summary

Link	Type	Maximum Flow LPS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
15	CONDUIT	164.55	0 01:05	2.33	2.39	1.00
16	CONDUIT	190.23	0 01:06	2.69	2.81	1.00
19	CONDUIT	466.75	0 01:16	2.93	2.59	1.00
2	CONDUIT	75.96	0 02:20	1.18	0.38	0.42
3	CONDUIT	83.27	0 01:05	1.18	1.23	1.00
4	CONDUIT	242.74	0 01:06	3.43	3.56	1.00
5	CONDUIT	465.04	0 01:08	2.92	2.33	1.00
6	CONDUIT	677.14	0 01:10	2.39	2.53	1.00

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 250-year Design Storm Event – Proposed Conditions PCSWMM Output

7	CONDUIT	1235.81	0	01:11	3.45	3.37	1.00
8	CONDUIT	1902.16	0	01:10	5.32	3.63	1.00
10	WEIR	561.92	0	01:10			0.50
11	WEIR	349.58	0	01:10			0.36
12	WEIR	0.00	0	00:00			0.00
13	WEIR	211.26	0	01:10			0.26
14	WEIR	0.00	0	00:00			0.00
17	WEIR	603.89	0	01:10			0.63
18	WEIR	605.00	0	01:10			0.61
20	WEIR	311.85	0	01:10			0.34
9	WEIR	247.35	0	01:10			0.29
1	DUMMY	22.91	0	01:07			

 Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class		Up		Down		Norm Ltd	Inlet Ctrl	
		Dry	Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit			
15	1.00	0.00	0.00	0.00	0.06	0.00	0.00	0.94	0.04	0.00
16	1.00	0.00	0.00	0.00	0.12	0.00	0.00	0.88	0.05	0.00
19	1.00	0.00	0.00	0.00	0.25	0.00	0.00	0.74	0.07	0.00
2	1.00	0.95	0.00	0.00	0.00	0.00	0.00	0.05	0.00	0.00
3	1.00	0.00	0.00	0.00	0.04	0.00	0.00	0.96	0.02	0.00
4	1.00	0.00	0.00	0.00	0.16	0.00	0.00	0.84	0.05	0.00
5	1.00	0.00	0.00	0.00	0.21	0.00	0.00	0.79	0.05	0.00
6	1.00	0.00	0.00	0.00	0.29	0.00	0.00	0.71	0.01	0.00
7	1.00	0.00	0.00	0.00	0.32	0.00	0.00	0.68	0.01	0.00
8	1.00	0.00	0.00	0.00	0.32	0.00	0.00	0.68	0.02	0.00

 Conduit Surge Summary

Conduit	Hours Full		Hours Above Full		Hours Capacity Limited
	Both Ends	Upstream	Dnstream	Normal Flow	
15	1.02	1.03	1.33	0.42	0.37
16	1.48	1.51	3.22	0.54	0.48
19	9.45	9.45	11.86	0.36	0.36
3	0.91	0.91	0.99	0.02	0.02
4	3.66	3.74	6.74	0.86	0.74
5	5.48	5.48	8.11	0.34	0.33
6	11.86	11.86	12.77	0.26	0.26
7	12.84	12.85	13.88	0.39	0.37
8	14.15	14.18	15.01	0.38	0.33

Analysis begun on: Wed Jun 07 15:42:52 2023
 Analysis ended on: Wed Jun 07 15:43:04 2023
 Total elapsed time: 00:00:12

Appendix F: CDS Quality Control Unit Cut Sheets



**CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION
BASED ON THE RATIONAL RAINFALL METHOD
BASED ON A FINE PARTICLE SIZE DISTRIBUTION**



Project Name: Watson Farm	Engineer: Development Engineering
Location: Thorndale ON	Contact: Kyle Zehr, EIT
OGS #: 1	Report Date: 6-Jun-23
Area: 4.600 ha	Rainfall Station #: 195
Weighted C: 0.62	Particle Size Distribution: FINE
CDS Model: 3035	CDS Treatment Capacity: 108 l/s

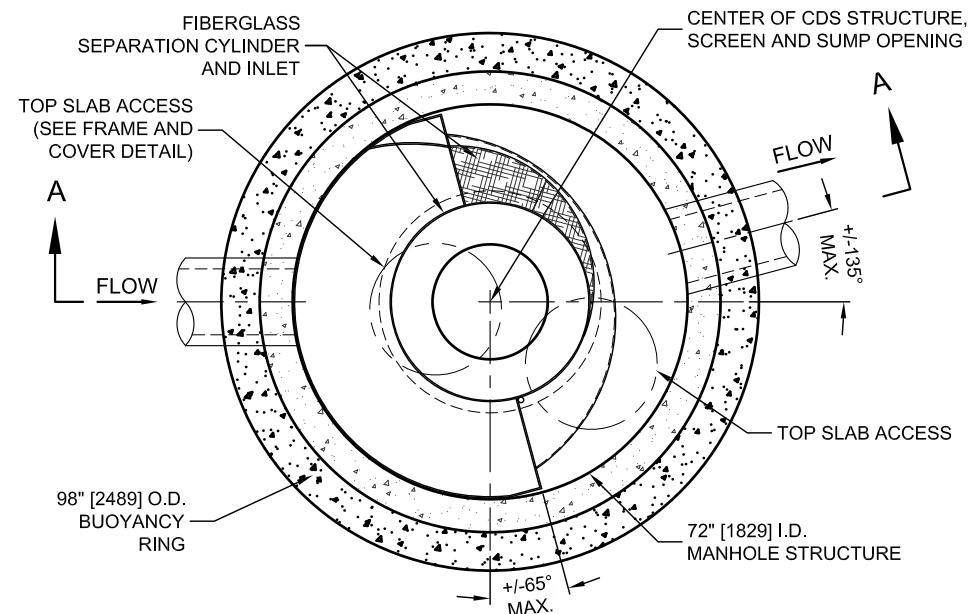
<u>Rainfall Intensity¹</u> <u>(mm/hr)</u>	<u>Percent Rainfall Volume¹</u>	<u>Cumulative Rainfall Volume</u>	<u>Total Flowrate (l/s)</u>	<u>Treated Flowrate (l/s)</u>	<u>Operating Rate (%)</u>	<u>Removal Efficiency (%)</u>	<u>Incremental Removal (%)</u>
1.0	10.3%	20.1%	7.9	7.9	7.3	96.8	9.9
1.5	9.7%	29.7%	11.8	11.8	11.0	95.7	9.2
2.0	8.9%	38.6%	15.7	15.7	14.6	94.7	8.4
2.5	7.7%	46.2%	19.7	19.7	18.3	93.6	7.2
3.0	6.5%	52.7%	23.6	23.6	21.9	92.6	6.0
3.5	4.2%	56.9%	27.5	27.5	25.6	91.5	3.9
4.0	4.7%	61.6%	31.5	31.5	29.2	90.5	4.2
4.5	3.9%	65.4%	35.4	35.4	32.9	89.4	3.4
5.0	3.4%	68.8%	39.3	39.3	36.5	88.4	3.0
6.0	4.7%	73.6%	47.2	47.2	43.8	86.3	4.1
7.0	4.6%	78.2%	55.1	55.1	51.2	84.2	3.9
8.0	3.5%	81.7%	62.9	62.9	58.5	82.1	2.9
9.0	2.3%	84.0%	70.8	70.8	65.8	80.0	1.8
10.0	2.6%	86.6%	78.6	78.6	73.1	77.9	2.0
15.0	6.7%	93.3%	118.0	107.6	100.0	64.0	4.3
20.0	2.7%	96.0%	157.3	107.6	100.0	48.0	1.3
25.0	1.7%	97.7%	196.6	107.6	100.0	38.4	0.7
30.0	1.3%	99.0%	235.9	107.6	100.0	32.0	0.4
35.0	0.6%	99.6%	275.3	107.6	100.0	27.4	0.2
40.0	0.3%	99.8%	314.6	107.6	100.0	24.0	0.1
45.0	0.0%	99.8%	353.9	107.6	100.0	21.3	0.0
50.0	0.2%	100.0%	393.2	107.6	100.0	19.2	0.0

86.5

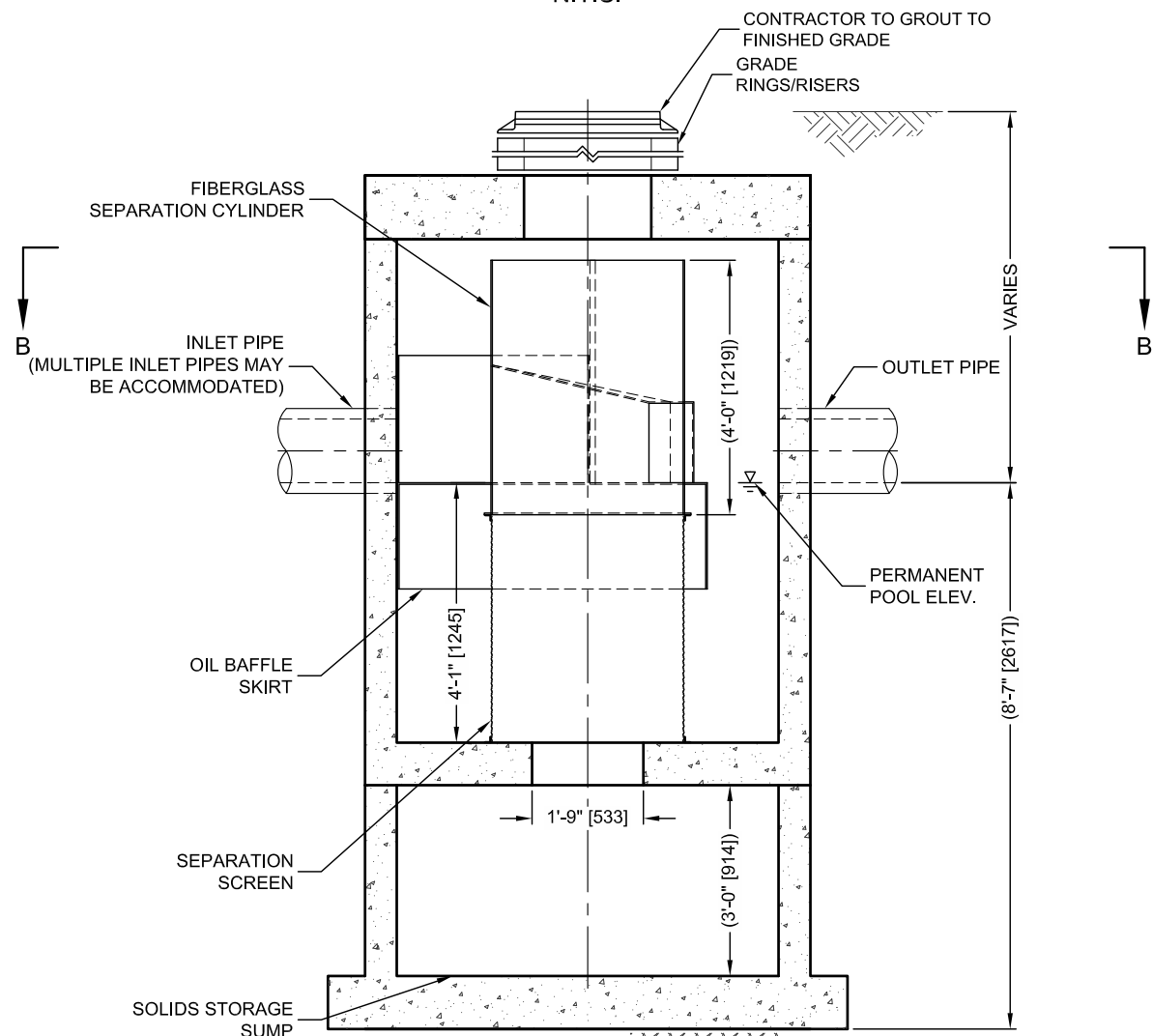
Removal Efficiency Adjustment² = 6.5%
Predicted Net Annual Load Removal Efficiency = 80.0%
Predicted Annual Rainfall Treated = 96.1%

1 - Based on 44 years of hourly rainfall data from Canadian Station 6144475, London ON
 2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.
 3 - CDS Efficiency based on testing conducted at the University of Central Florida
 4 - CDS design flowrate and scaling based on standard manufacturer model & product specifications

C:\USERS\HUDA.ECHELON\VIDEODOCUMENTS\START ITEMS\PMSU SAMPLE DRAWINGS\CDS3035-6-C-DTL.DWG 5/29/2022 11:43 PM



PLAN VIEW B-B
N.T.S.



ELEVATION A-A
N.T.S.



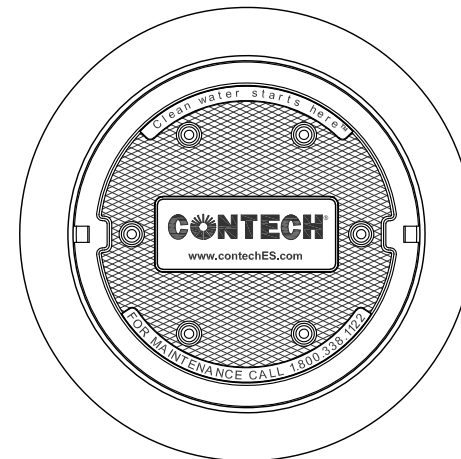
THIS PRODUCT MAY BE PROTECTED BY ONE OR MORE OF THE FOLLOWING U.S. PATENTS: 6,788,848; 6,841,722; 6,911,502; 6,981,783; RELATED FOREIGN PATENTS, OR OTHER PATENTS-PENDING.

CDS PMSU3035-6-C DESIGN NOTES

THE STANDARD CDS PMSU3035-6-C CONFIGURATION IS SHOWN. ALTERNATE CONFIGURATIONS ARE AVAILABLE AND ARE LISTED BELOW. SOME CONFIGURATIONS MAY BE COMBINED TO SUIT SITE REQUIREMENTS.

CONFIGURATION DESCRIPTION

- GRATED INLET ONLY (NO INLET PIPE)
- GRATED INLET WITH INLET PIPE OR PIPES
- CURB INLET ONLY (NO INLET PIPE)
- CURB INLET WITH INLET PIPE OR PIPES
- CUSTOMIZABLE SUMP DEPTH AVAILABLE
- ANTI-FLOTATION DESIGN AVAILABLE UPON REQUEST



FRAME AND COVER
(DIAMETER VARIES)
N.T.S.

SITE SPECIFIC DATA REQUIREMENTS

STRUCTURE ID				
WATER QUALITY FLOW RATE (CFS OR L/s)				*
PEAK FLOW RATE (CFS OR L/s)				*
RETURN PERIOD OF PEAK FLOW (YRS)				*
SCREEN APERTURE (2400 OR 4700)				*
PIPE DATA:	I.E.	MATERIAL	DIAMETER	
INLET PIPE 1	*	*	*	
INLET PIPE 2	*	*	*	
OUTLET PIPE	*	*	*	
RIM ELEVATION				*
ANTI-FLOTATION BALLAST	WIDTH	HEIGHT		
	*	*		
NOTES/SPECIAL REQUIREMENTS:				
* PER ENGINEER OF RECORD				

GENERAL NOTES

1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
2. DIMENSIONS MARKED WITH () ARE REFERENCE DIMENSIONS. ACTUAL DIMENSIONS MAY VARY.
3. FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. www.contechES.com
4. CDS WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING.
5. STRUCTURE SHALL MEET AASHTO HS20 AND CASTINGS SHALL MEET HS20 (AASHTO M 306) LOAD RATING, ASSUMING GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION.
6. PVC HYDRAULIC SHEAR PLATE IS PLACED ON SHELF AT BOTTOM OF SCREEN CYLINDER. REMOVE AND REPLACE AS NECESSARY DURING MAINTENANCE CLEANING.

INSTALLATION NOTES

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE CDS MANHOLE STRUCTURE (LIFTING CLUTCHES PROVIDED).
- C. CONTRACTOR TO ADD JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS, AND ASSEMBLE STRUCTURE.
- D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT PIPES. MATCH PIPE INVERTS WITH ELEVATIONS SHOWN.
- E. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.



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**CDS PMSU3035-6-C
INLINE CDS
STANDARD DETAIL**